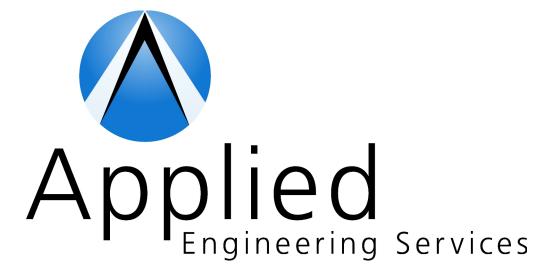
PORTER COUNTY OFFICE BUILDING

PORTAGE, IN 100% CONSTRUCTION DOCUMENTS - BP1 08.17.18









PROJECT TEAM:

PORTER COUNTY BOARD OF COMMISSIONERS 155 INDIANA AVENUE, SUITE 205 VALPARAISO, IN 46383

SCOTT McCLURE

CIVIL DESIGN: AMERICAN STRUCTUREPOINT, INC. 7260 SHADELAND STATION INDIANAPOLIS, IN 46256 317.547.5580 317.543.0270 DKMIECIK@STRUCTUREPOINT.COM DAN KMIECIK PE

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ROY WAGNER, CxA

CONSTRUCTION MANAGER THE SKILLMAN CORPORATION 8006 AETNA STREET MERRILLVILLE, IN 46410 219.649.7030

DMANDERSON@SKILLMAN.COM DEREK ANDERSON

CODE ANALYSIS: RTM CONSULTANTS, INC 6640 PARKDALE PLACE, SUITE J INDIANAPOLIS, IN 46254 317.329.7700, X106 COLLESTER@RTMCONSULTANTS.COM

CHRISTINA COLLESTER

ABBREVIATIONS							SHEET INDEX				
A		G		ORIG	ORIGINAL	SHEET NUMBER	SHEET NAME	CURR REV	REV DATE	REV DESCR	
A/E	ARCHITECT/ENGINEER	GALV	GALVANIZED			GENERAL					
ACOUS	ACOUSTICAL	GLZ	GLAZING	Р		G000	COVER SHEET				
ACT	ACOUSTICAL CEILING	GUT	GUTTER	PERM	PERMANENT	G001	SHEET INDEX				
7.0.	TILE	GYP BD/GWB		PH	PHASE						
ADA	AMERICAN'S WITH	OTT BB/OVE	011 00W 207 W 2	PL	PLATE	CIVIL					
	DISABILITIES ACT	Н		PLAM	PLASTIC LAMINATE	C002	GENERAL NOTES				
ADD	ADDENDUM	HC	HANDICAP(PED)	PLAS	PLASTIC	C100	OVERALL EXISTING TOPOGRAPHY PLAN				
ADJ	ADJACENT	HCW	HOLLOW CORE WOOD	PNL	PANEL	C110	DEMOLITION PLAN				
AFF	ABOVE FINISH FLOOR	HDWE/HDW	HARDWARE	PREFAB	PREFABRICATED	C200	OVERALL SITE PLAN				
ALT	ALTERNATE					C201	SITE PLAN				
ALUM	ALUMINUM	HM	HOLLOW METAL DOOR	PREFIN	PREFINISHED	C301	GRADING PLAN				
ANOD	ANODIZED	HMD	HOLLOW METAL DOOR	PRELIM	PRELIMINARY	C302	FLOOD ROUTING PLAN				
ARCH	ARCHITECT (URAL)	HORIZ	HORIZONTAL	PREP	PREPARATION	C310	STORM SEWER PLAN AND PROFILES				
AROH	AROHITEOT (ORAE)	HT	HEIGHT	PROJ	PROJECT	C311	STORM SEWER PLAN AND PROFILES				
D		HVAC	HEATING,	PT	PRESERVATIVE	C312	STORM SEWER PLAN AND PROFILES				
В	DI III DINIC		VENTILATION, AIR CONDITIONING		TREATED						
BLDG	BUILDING		CONDITIONING	PTD	PAINTED	C313	STORM SEWER PLAN AND PROFILES AND DATA TABLES				
BLK	BLOCK			PTN	PARTITION	C401	UTILITY PLAN				
BLKG	BLOCKING					C501	EROSION CONTROL PLAN				
BM	BEAM	ICF	INSULATED	Q		C510	STORM WATER POLLUTION PREVENTION PLAN				
BOT	BOTTOM		CONCRETE FORM	QT	QUARRY TILE	C520	EROSION CONTROL DETAILS				
BRG	BEARING	ID	INSIDE DIAMETER			C521	EROSION CONTROL DETAILS				
		IF	INSIDE FACE	R		C600	SITE DETAILS				
С		INCAND	INCANDESCENT	R	RADIUS	C601	SITE DETAILS				
C/C	CENTER TO CENTER	INDIC	INDICATE	RD	ROOF DRAIN	C602	SITE DETAILS				
CAB	CABINET	INSTL	INSTALL(ATION)	REC	RECESSED	C610	CITY OF PORTAGE STANDARD DETAILS				
CB	CORNER BEAD	INSUL	INSULATION	RECPT	RECEPTACLE	C611	IAWC WATER INSTALLATION DETAILS AND NOTES	+			
CER	CERAMIC	INT	INTERIOR	REF		C612	IAWC WATER INSTALLATION DETAILS AND NOTES				
CF	CUBIC FOOT				REFER(ENCE)	L100	LANDSCAPE PLAN				
	CONTRACTOR	1		REINF	REINFORCE(D)(ING)(M	L200	LANDSCAPE DETAILS				
CFCI	FURNISHED /	JST	JOIST	DEOD	ENT)	L200	LANDSCAPE DETAILS				
	CONTRACTOR	JT	JOINT	REQD	REQUIRED	OTDUOTUDAL					
	INSTALLED	J 1	30111	RESIL	RESILIENT	STRUCTURAL					
CHAN	CHANNEL	1		RM	ROOM	S001	ABBREVIATIONS AND SYMBOLS				
CJ	CONTROL JOINT	L LANZ	LANZATORY	RO	ROUGH OPENING	S002	GENERAL NOTES				
CL	CENTER LINE	LAV	LAVATORY	RTU	ROOF TOP UNOT	S003	SPECIAL INSPECTION REQUIREMENTS				
CLG	CEILING	LD BRG	LOAD BEARING			S004	LOAD MAPS				
CMU	CONCRETE MASONRY	LF	LINEAR FOOT	S		S101	FOUNDATION PLAN				
OIVIO	UNIT	LTG	LIGHTING	SAN	SANITARY	S131	ROOF FRAMING PLAN				
CONC	CONCRETE	LTWT	LIGHTWEIGHT	SCW	SOLID CORE WOOD	S301	FOUNDATION SCHEDULES, SECTIONS, AND DETAILS				
CONSTR	CONSTRUCTION	LVR	LOUVER	SF	SQUARE FOOT	S302	FOUNDATION SECTIONS AND DETAILS				
CONT	CONTINUOUS			SPKLR	SPRINKLER	S501	STEEL SCHEDULES, SECTIONS, AND DETAILS				
CP/CPT	CARPET	M		SS	STAINLESS STEEL	S511	STEEL COLUMN AND BASE PLATE SCHEDULES				
OF/OF I	CARFET	MAS	MASONRY	STD	STANDARD	S521	STEEL FRAME ELEVATIONS, SECTIONS AND DETAILS				
D		MATL	MATERIAL	STL	STEEL						
D	DEMOLITION (IOLI)	MAX	MAXIMUM	STOR or ST	STORAGE	ARCHITECTURAL					
DEMO	DEMOLITION (ISH)	MECH	MECHANICAL	STRUCT	STRUCTURAL	A001	WALL TYPES				
DF	DRINKING FOUNTAIN	MED	MEDICINE (MEDICAL)	SUSP	SUSPENDED	A101	FIRST FLOOR PLAN/RCP				
DIFF	DIFFUSER	MET	METAL	SYM	SYMBOL	A131	ROOF PLAN	+			
DIM	DIMENSION	MFG	MANUFACTURER (ING)	SYS	SYSTEM	A201	EXTERIOR ELEVATIONS	+			
DISP	DISPENSER	MIN	MINIMUM	5.5	J . J . L.W.	A321	WALL SECTIONS	+			
DO	DOOR OPENING	MISC	MISCELLANEOUS	т		A322	WALL SECTIONS	+			
DS	DOWNSPOUT	MLWK	MILLWORK	T&G	TONGUE AND	A411	DUMPSTER ENCLOSURE				
DWG	DRAWING	MO	MASONRY OPENING	ING	GROOVE	l	PLAN DETAILS				
		MTD	MOUNTED	T/	TOP OF	A501					
E		MTG	MOUNTING	TEL	TELEPHONE	A521	SECTION DETAILS				
EJ	EXPANSION JOINT					A601	DOOR SCHEDULES				
ELEC	ELECTRICAL	MULL	MULLION	TEMP	TEMPORARY						
	(ELEVATOR)	MULT	MULTIPLE	THRES	THRESHOLD						
ELEV	ELEVATION			TOT	TOTAL						
ENCL	ENCLOSURE	N		TYP	TYPICAL						
ENGR	ENGINEER	N/A	NOT APPLICABLE								
EQUIP	EQUIPMENT	NIC	NOT IN CONTRACT	U							
EWC	ELECTRICAL WATER	NOM	NOMINAL	UNIF	UNIFORM						
	COOLER	NTS	NOT TO SCALE	UNO	UNLESS NOTED						
EXST	EXISTING				OTHERWISE						
L/(0)	2.101110	0		UR	URINAL						
F		0/0	OUT TO OUT								
F F/F	FACE TO FACE	OA	OVERALL	V							
	FOUNDATION	OC	ON CENTER	VB	VINYL BASE						
FDN		OD	OUTSIDE DIAMETER	VCT	VINYL COMPOSITION						
FE	FIRE EXTINGUISHER	OF	OUTSIDE FACE		TILE						
FHC	FIRE HOSE CABINET	0 1	JOINDE I AUL	\/EDT	VEDTICAL	1					

VERTICAL

VINYL TILE

VINYL WALL

COVERING

WOOD

WINDOW

WEIGHT

WALL HUNG

WALL TO WALL

WATER CLOSET

SHEET INDEX SHEET NUMBER SHEET NAME CURR REV REV DATE REV DESCR

GENERAL NOTES

- A. SEE SHEET G001 FOR GRAPHIC SYMBOL LEGEND.
- B. SEE SHEET A001 FOR GENERAL PARTITION NOTES.
- C. SEE SHEET A701 FOR GENERAL FINISH NOTES D. COORDINATE WORK OF ALL TRADES PRIOR TO STARTING CONSTRUCTION. E. ALL DIMENSIONS ARE TO GRID LINES, FACE OF STUD FRAMING, FINISHED FACE OF EXISTING WALL SURFACE, OR FACE OF CONCRETE / MASONRY UNITS UNLESS NOTED
- OTHERWISE. F. THE CONTRACTOR AND SUBCONTRACTORS INVOLVED IN THIS PROJECT SHALL BE RESPONSIBLE FOR DESIGNING AND INSTALLING THEIR RESPECTIVE WORK AND SYSTEMS TO MEET ALL APPLICABLE FEDERAL, STATE, AND LOCAL CODES, LAWS, SAFETY REGULATIONS, HAZARDOUS WASTE LAWS, ETC. THE CONTRACTOR SHALL FURNISH ALL NECESSARY PERMITS.
- G. THE CONTRACTOR SHALL REVIEW THE DRAWINGS AND UNDERSTAND THE SCOPE OF THE DRAWINGS TO BE THE FOLLOWING: THESE CONSTRUCTION DOCUMENTS (DRAWINGS, SPECIFICATIONS, ADDENDA, ETC.) INDICATE THE GENERAL SCOPE OF THE PROJECT IN TERMS OF ARCHITECTURAL DESIGN CONCEPTS, THE DIMENSIONS OF THE BUILDING, THE MAJOR ARCHITECTURAL ELEMENTS, AND THE MAJOR STRUCTURAL, MECHANICAL, AND ELECTRICAL SYSTEMS. THE DOCUMENTS DO NOT AND ARE NOT INTENDED TO INDICATE OR DESCRIBE IN DETAIL ALL THE NECESSARY WORK REQUIRED FOR FULL PERFORMANCE OF, AND COMPLETION OF THE REQUIREMENTS OF THE CONTRACT. ON THE BASIS OF THE GENERAL SCOPE INDICATED IN THESE DOCUMENTS, THE TRADE CONTRACTORS SHALL FURNISH ALL ITEMS REQUIRED FOR THE PROPER EXECUTION AND COMPLETION OF THEIR WORK. ALL WORK SHALL BE COMPLETE IN EVERY DETAIL AND THE CONTRACTORS SHALL PROVIDE A ONE YEAR WARRANTY FOR THEIR WORK.
- H. CONTRACTORS SHALL FOLLOW AND OBEY ALL FEDERAL, STATE AND LOCAL CODES, LAWS, SAFETY REGULATIONS AND HAZARDOUS WASTE LAWS, ETC. I. DO NOT SCALE DRAWINGS. DIMENSIONS SHALL GOVERN EACH BUILDING COMPONENT
- LOCATION. BRING ANY DISCREPANCIES TO THE ARCHITECT'S ATTENTION IN WRITING J. BUILDING FIRST FLOOR IS REFERENCE ELEVATION 100'-0". REFERENCE CIVIL DRAWINGS
- FOR U.S.G.S. ELEVATION. K. ALL WORK SHALL BE PERFORMED IN A SKILLED WORKMANSHIP TYPE AND MANNER ACCEPTABLE TO THE ARCHITECT AND OWNER.
- L. CAULK INTERSECTION BETWEEN DIFFERENT MATERIALS. M. CAULK TOILET FIXTURES AND COUNTERTOP SPLASHES TO FINISH SURFACE IN ACCORDANCE WITH SEALANT SCHEDULE IN SPECIFICATIONS.
- N. EXISTING CONDITIONS SHALL BE FIELD VERIFIED PRIOR TO BIDDING OR BEGINNING OF WORK. ANY ERRORS OR OMISSIONS SHALL BE BROUGHT TO THE ARCHITECT'S
- ATTENTION IN WRITING IMMEDIATELY.
- O. COVER ALL RETURN AIR AND EXHAUST GRILLES WITH FILTER MEDIA FOR DURATION OF JOB AND CHANGE REGULARLY.



PORTER COUNTY OFFICE

STRUCTUREPOINT

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SKILLMAN V

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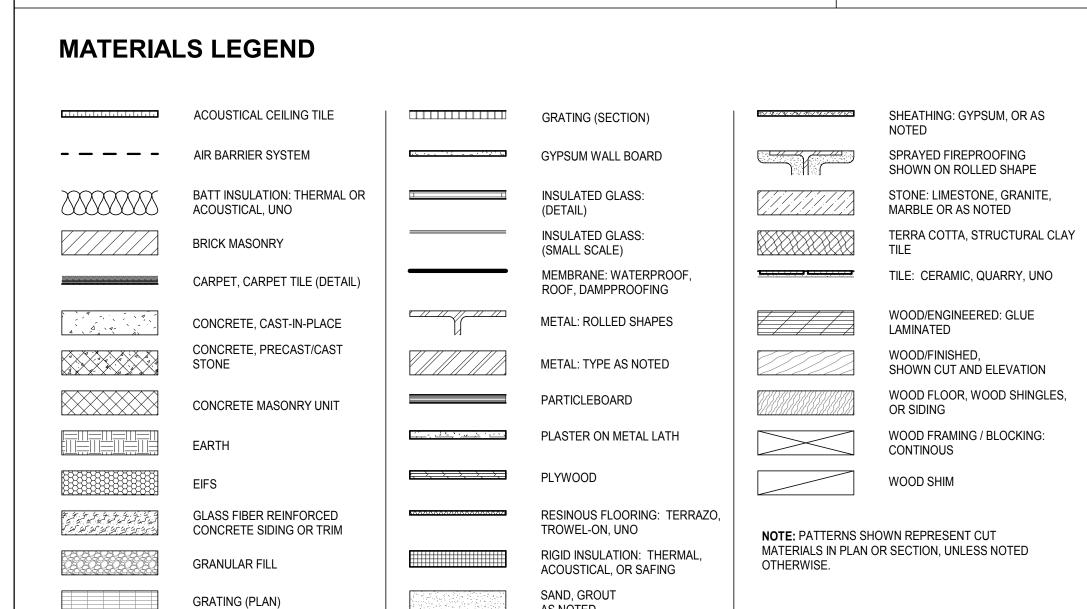
BUILDING

PORTAGE, IN

CERTIFIED BY

ISSUANCE INDEX 08.17.18 PROJECT PHASE: 100% CONSTRUCTION DOCUMENTS - BP1

REVISION SCHEDULE NO. DESCRIPTION DATE



AS NOTED

CONTRACTOR

INSTALLED

OVERHEAD

OPENING

OPPOSITE

OPTIONAL

OVERFLOW ROOF

OWNER FURNISHED -

FINISHED FLOOR

FLUORESCENT

FACE OF FINISH

FACE OF STUD

FIRE RATING

FIRE TREATED

FOOTING

FURNITURE

FURRING

FACE OF CONCRETE

FACE OF MASONRY

FLOOR(ING)

FINISH

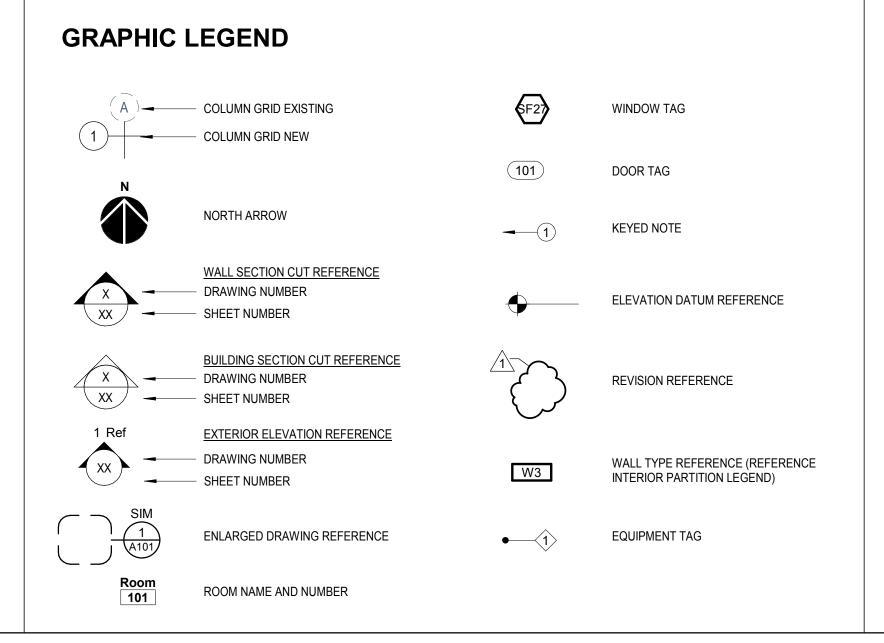
OFCI

FIN FL

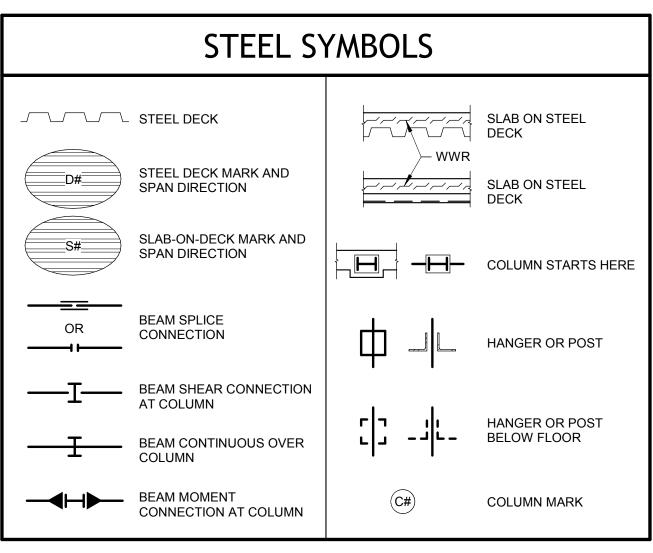
FOC

FOS

FLR (FLRG)



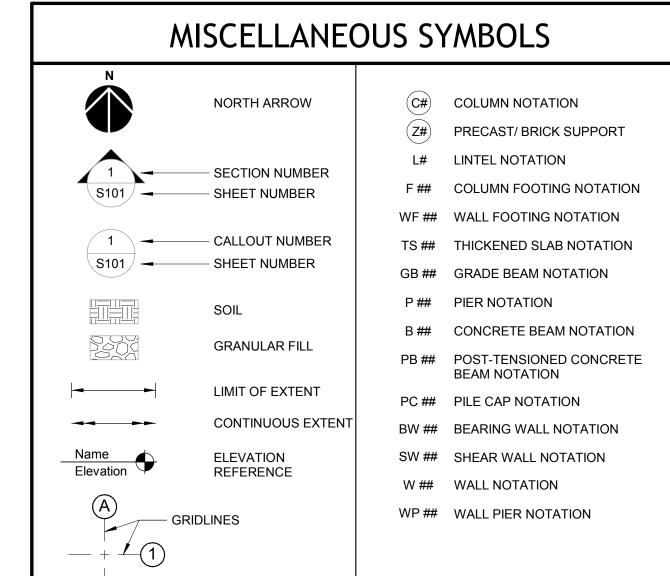
Project Number 2017.01279 SHEET INDEX

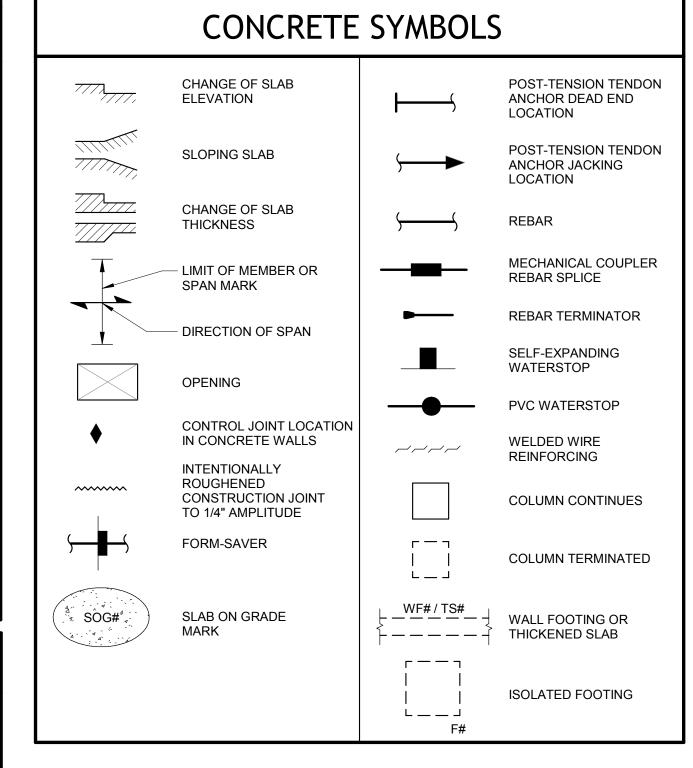


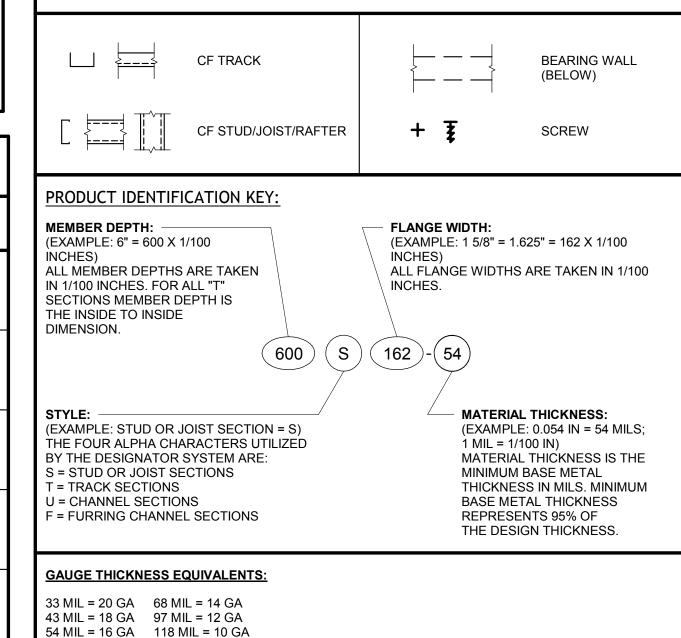
	STEEL M	EMBERS	
SHAPE	SECTION	ELEVATION	PLAN VIEW
W-SHAPE BEAM			
CHANNEL			
ANGLE			
DOUBLE ANGLE	LLV LLH		
HOLLOW STRUCTURAL SECTION -RECTANGULAR			
HOLLOW STRUCTURAL SECTION -CIRCULAR (PIPE)			
OPEN WEB STEEL JOIST			

MASONRY MEMBERS							
	CMU SECTION	•	SINGLE REINFORCEMENT (FULLY GROUT REINFORCED CELLS)				
	CMU BOND BEAM SECTION (FULLY GROUTED)	PLAN •	DOUBLE REINFORCEMENT				
	CMU LINTEL SECTION (FULLY GROUTED)	PLAN	(FULLY GROUT REINFORCED CELLS)				

CONNECTORS							
CONNECTOR	SECTION		END/ALT VIEW				
CAST-IN ANCHOR ROD		<u>‡</u>	0				
POST-INSTALLED MECHANICAL ANCHOR	>	—	0				
POST INSTALLED ADHESIVE ANCHOR		+	0				
HEADED STUD			0				
BOLT	+	+	0				







COLD-FORMED STEEL SYMBOLS



STRUCTURAL DRAWINGS ABBREVIATIONS

ARCHITECT/ENGINEER

AMERICAN CONCRETE INSTITUTE

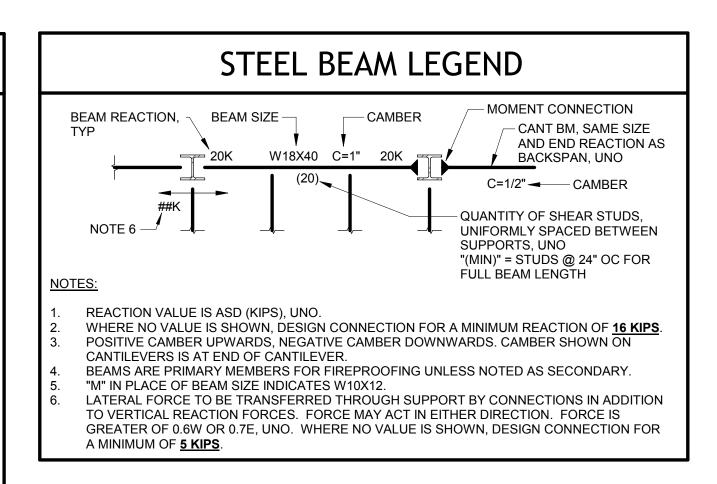
J/BRG

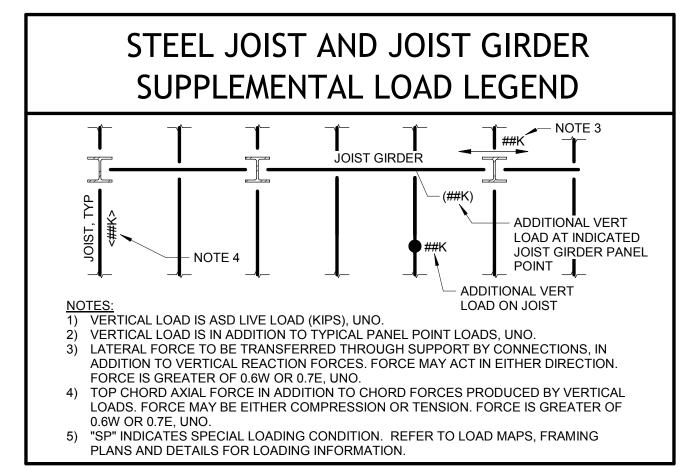
JST

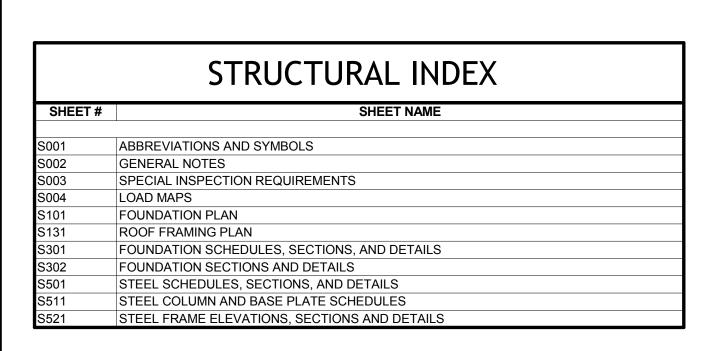
JOIST BEARING

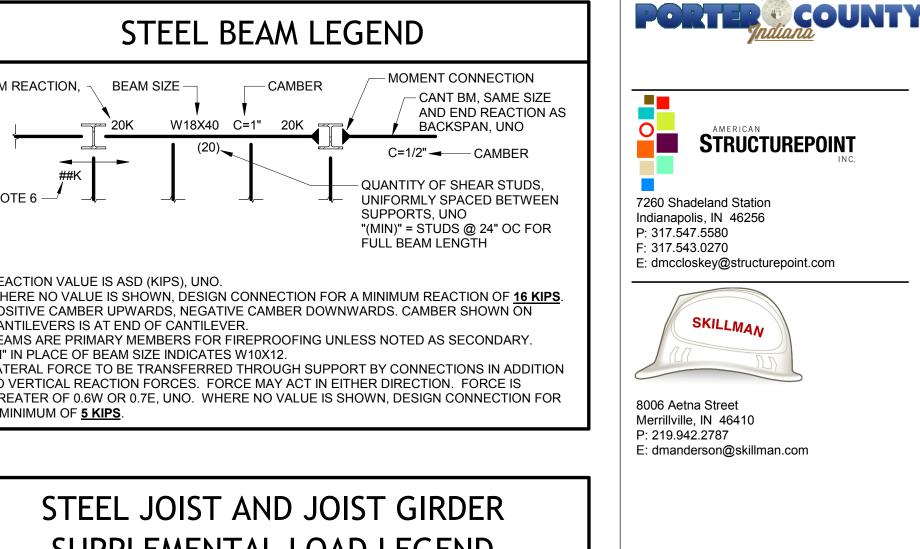
JOIST

JOINT









PORTER COUNTY -TRUSTEES OFFICE

PORTAGE, IN

CERTIFIED BY

ISSUANCE INDEX 08.20.18 PROJECT PHASE:

REVISION SCHEDULE NO. DESCRIPTION DATE

100% CONSTRUCTION DOCUMENTS - BP1

Project Number 2017.01279

ABBREVIATIONS AND SYMBOLS

STRUCTURES INDICATED ON THESE DRAWINGS HAVE BEEN DESIGNED FOR THE IN-SERVICE LOADS ONLY. THE METHODS, MEANS, PROCEDURES, AND SEQUENCES OF CONSTRUCTION ARE THE RESPONSIBILITY OF THE CONTRACTOR. THE STRUCTURES ARE DESIGNED TO BE SELF-SUPPORTING AND STABLE AFTER CONSTRUCTION IS COMPLETE. IT IS THE CONTRACTOR'S RESPONSIBILITY TO DETERMINE CONSTRUCTION PROCEDURES AND SEQUENCE TO ENSURE SAFETY OF THE STRUCTURES AND RELATED COMPONENTS DURING CONSTRUCTION. THIS INCLUDES THE ADDITION OF NECESSARY SHORING, SHEETING, TEMPORARY BRACING, GUYS, TIEDOWNS, ETC.

IT IS THE CONTRACTOR'S RESPONSIBILITY TO VERIFY THAT CONSTRUCTION LOADS DO NOT EXCEED THE CAPACITY OF ANY STRUCTURAL ELEMENT AT THE TIME THE LOADS ARE APPLIED, INCLUDING BUT NOT LIMITED TO: WEIGHTS OF MATERIALS, WEIGHTS OF EQUIPMENT AND WORKERS, AND ALL LOADS APPLIED FROM TEMPORARY LIFTS, HOISTS AND CRANES, ETC.

WITH THE DRAWINGS AND SPECIFICATIONS OF ALL OTHER DISCIPLINES, TRADES, AND DELEGATED DESIGN ELEMENTS. THE CONTRACTOR IS RESPONSIBLE FOR COORDINATION OF THE STRUCTURAL WORK WITH ALL OTHER APPLICABLE TRADES.

THE STRUCTURAL DRAWINGS AND SPECIFICATIONS SHALL BE USED IN CONJUNCTION

THE GENERAL NOTES ON THE DRAWINGS ARE TO BE USED IN CONJUNCTION WITH THE FULL WRITTEN MATERIAL SPECIFICATIONS (IF ANY) FOR THE PROJECT. DO NOT SCALE THE DRAWINGS. REFER TO THE WRITTEN DIMENSIONS AND

IF A DISCREPANCY IS NOTED ON THE DRAWINGS, GENERAL NOTES, OR SPECIFICATIONS. THE CONTRACTOR SHALL SUBMIT TO THE STRUCTURAL ENGINEER A WRITTEN REQUEST FOR CLARIFICATION AND SHALL NOT PROCEED WITH THE AFFECTED WORK WITHOUT DOCUMENTED RESOLUTION OF THE DISCREPANCY. ALL COSTS RESULTING FROM THE CONTRACTOR IMPROPERI Y PROCEEDING WITH THE AFFECTED WORK PRIOR TO DOCUMENTED RESOLUTION OF THE DISCREPANCY. INCLUDING COST OF REMOVAL AND REPLACEMENT OF NON-CONFORMING WORK, SHALL BE BORNE BY THE CONTRACTOR AT NO ADDITIONAL COST TO THE OWNER.

PENETRATIONS AND EMBEDDED ITEMS

CONTRACTOR SHALL COORDINATE WITH ALL AFFECTED TRADES THE REQUIRED SIZES. TYPES, AND LOCATIONS OF ALL EMBEDDED ITEMS IN, AND ALL PENETRATIONS THROUGH, STRUCTURAL ELEMENTS PRIOR TO CONSTRUCTION.

NO PENETRATIONS THROUGH STRUCTURAL ELEMENTS, OTHER THAN THOSE SHOWN ON THE STRUCTURAL DRAWINGS, SHALL BE MADE WITHOUT THE APPROVAL OF THE STRUCTURAL ENGINEER.

NO ITEMS (INCLUDING, BUT NOT LIMITED TO, CONDUIT, PIPING, ETC.), OTHER THAN THOSE SHOWN ON THE STRUCTURAL DRAWINGS, SHALL BE EMBEDDED IN STRUCTURAL ELEMENTS WITHOUT THE APPROVAL OF THE STRUCTURAL ENGINEER.

ALL EMBEDDED ITEMS IN EXPOSED EXTERIOR CONCRETE SHALL BE GALVANIZED, PLASTIC, OR EPOXY-COATED.

CONCRETE MIX DESIGN SUBMITTAL

THE CONTRACTOR SHALL SUBMIT FOR THE REVIEW OF THE STRUCTURAL ENGINEER A MIX DESIGN FOR EACH PROPOSED CLASS OF CONCRETE. EACH MIX DESIGN SHALL BE IDENTIFIED BY A MIX NUMBER OR OTHER UNIQUE IDENTIFICATION. THE CONTRACTOR SHALL NOT VARY FROM THE MIX DESIGNS NOR USE ANY CONCRETE OTHER THAN THE APPROVED MIX DESIGNS WITHOUT THE APPROVAL OF THE STRUCTURAL ENGINEER. MIX DESIGN SUBMITTALS SHALL INCLUDE THE FOLLOWING INFORMATION:

- 1. MIX DESIGN NUMBER OR UNIQUE IDENTIFICATION AND INTENDED LOCATION OF PLACEMENT CEMENT TYPE, PROPORTION AND NAME OF MANUFACTURER.
- FLY ASH PROPORTION (WHEN USED), LABORATORY ANALYSIS CERTIFICATION, AND NAME AND LOCATION OF SUPPLIER.
- 4. COURSE AGGREGATE PROPORTION, GRADATION REPORT, NAME AND LOCATION OF SUPPLIER.

5. FINE AGGREGATE PROPORTION, GRADATION REPORT, NAME AND LOCATION

- OF SUPPLIER. 6. MIXING WATER PROPORTION AND SOURCE.
- ADMIXTURE DOSAGES, PRODUCT NAME(S) AND MANUFACTURER NAME(S)
- 8. FIBER REINFORCEMENT DOSAGE (WHEN USED), PRODUCT NAME AND MANUFACTURER NAME
- 9. DESIGN 28-DAY COMPRESSIVE STRENGTH (F'C). 10. DESIGN SI UMP RANGE.
- 11. DESIGN AIR-ENTRAINMENT (FOR CONCRETE REQUIRING ENTRAINED AIR). 12. STATISTICAL ANALYSIS OF LABORATORY STRENGTH TEST DATA IN ACCORDANCE WITH "STANDARD DEVIATION" DETERMINATION OUTLINED IN

DELEGATED DESIGN

FOR ALL SUBMITTALS INDICATED AS "DELEGATED DESIGN." THE CONTRACTOR SHALL ENGAGE A SPECIALTY STRUCTURAL ENGINEER LICENSED IN THE STATE WHERE THE PROJECT IS LOCATED TO DESIGN AND DETAIL THE ITEMS NOTED IN THE STRUCTURAL SUBMITTALS AND SPECIFICATIONS AS A DELEGATED DESIGN.

DELEGATED DESIGN CALCULATIONS AND RELATED DRAWINGS SHALL CONTAIN THE FOLLOWING, AS A MINIMUM:

- 1. COVER PAGE SIGNED AND SEALED BY THE SPECIALTY STRUCTURAL ENGINEER INCLUDING A STATEMENT OF CERTIFICATION THAT THE SUBMITTED CALCULATIONS ARE IN CONFORMANCE WITH THE DESIGN CRITERIA PROVIDED IN THE CONTRACT DOCUMENTS AND THAT THE RELATED SHOP DRAWINGS ARE IN
- CONFORMANCE WITH THE SUBMITTED CALCULATIONS. TABLE OF CONTENTS, PLACED ON, OR IMMEDIATELY FOLLOWING THE COVER
- 3. SUMMARY OF APPLICABLE CODE CRITERIA, LOAD DATA AND PERFORMANCE CRITERIA AS OUTLINED IN THE CONTRACT DOCUMENTS.
- 4. CLEAR DEFINITION OF THE LOCATION(S) IN THE STRUCTURE WHERE EACH CALCULATION APPLIES LOCATION, TYPE, MAGNITUDE AND DIRECTION OF ALL LOADS IMPOSED ON THE
- SUBMITTALS THAT DO NOT CONTAIN THE INFORMATION NOTED ABOVE WILL BE REJECTED WITHOUT COMMENT.

STRUCTURE BY THE DELEGATED DESIGN SYSTEM/COMPONENTS.

THE STRUCTURAL ENGINEER'S REVIEW OF DELEGATED DESIGN SUBMITTALS WILL BE

FOR GENERAL CONFORMANCE WITH THE DESIGN LOADING, DESIGN INTENT AND LOADS

SHOP DRAWING SUBMITTALS

THE CONTRACTOR SHALL PREPARE DETAILED SHOP DRAWINGS TO ENABLE ALL PARTS OF THE WORK TO BE FABRICATED AND CONSTRUCTED IN ACCORDANCE WITH THE DRAWINGS AND SPECIFICATIONS. THESE SHOP DRAWINGS WILL BE REVIEWED FOR GENERAL COMPLIANCE WITH THE DESIGN INTENT ONLY. THE CONTRACTOR IS RESPONSIBLE FOR ALL DIMENSIONS, ACCURACY AND FIT OF WORK.

ALL SHOP DRAWINGS SHALL BE REVIEWED BY THE CONTRACTOR PRIOR TO SUBMITTAL TO THE STRUCTURAL ENGINEER. DRAWINGS SUBMITTED WITHOUT CONTRACTOR'S REVIEW WILL BE RETURNED UNCHECKED.

SUBMIT SHOP DRAWINGS FOR EACH OF THE FOLLOWING ITEMS:

PROVIDE ALL SUBMITTALS IN ELECTRONIC PDF FORMAT

- CONCRETE REINFORCEMENT
- STRUCTURAL STEEL STEEL JOISTS AND JOIST GIRDERS (INCLUDING ALL BRIDGING AND BRACING)
- SUBMIT DELEGATED DESIGN SHOP DRAWINGS FOR EACH OF THE FOLLOWING ITEMS: 1. STRUCTURAL STEEL CONNECTIONS (INCLUDING DESIGN CALCULATIONS AND SUMMARY PAGE FOR CONNECTIONS NOT SPECIFICALLY DETAILED IN THE
- CONTRACT DRAWINGS) 2. GLAZING SYSTEMS - CURTAIN WALL AND STOREFRONT (INCLUDING DESIGN CALCULATIONS AND CONNECTION DETAILS)
- 3. COLD-FORMED STEEL FRAMING (INCLUDING DESIGN CALCULATIONS, MEMBER PROPERTIES, FASTENER REQUIREMENTS, ASSEMBLY DETAILS AND **CONNECTION DETAILS)**

PRODUCT DATA SUBMITTALS

THE CONTRACTOR SHALL SUBMIT FOR APPROVAL PRODUCT DATA FOR THE SPECIFIC ITEMS LISTED BELOW. CONTRACTOR SHALL NOT USE PRODUCTS OTHER THAN THOSE SUBMITTED WITHOUT THE APPROVAL OF THE STRUCTURAL ENGINEER. SUBMIT HARD COPIES OR ELECTRONIC VERSIONS OF PRODUCT DATA. FOR HARD

COPY OPTION. SUBMIT A MINIMUM OF TWO COPIES OF PRODUCT DATA TO THE STRUCTURAL ENGINEER FOR REVIEW (ONE COPY SHALL BE RETAINED BY THE STRUCTURAL ENGINEER). FOR ELECTRONIC OPTION, SUBMIT PRODUCT DATA IN ADOBE PDF FORMAT.

- FIBER REINFORCEMENT FOR CONCRETE CONCRETE CURING COMPOUND
- 3. CONCRETE JOINT SEALANT 4. EXPANSION ANCHORS
- 5. ADHESIVE ANCHORS NON-SHRINK GROUT
- 7. COLD-FORMED STEEL FRAMING 8. COLD-FORMED STEEL FRAMING CONNECTOR HARDWARE VAPOR RETARDER

FOUNDATIONS

RELATED WORK.

SEE GEOTECHNICAL REPORT NUMBER 18SB0022 PREPARED BY ALT & WITZIG ENGINEERING, INC DATED 30TH MARCH 2018 AND ALL APPLICABLE ADDENDA AND AMENDMENTS FOR INFORMATION RELATED TO FOUNDATION EXCAVATIONS AND SOIL-

DESIGN NET SOIL PRESSURE:

1500 PSF SPREAD FOOTINGS: CONTINUOUS WALL FOOTINGS: 1500 PSF

FOUNDATIONS AND SOILS RELATED WORK SHALL BE INSPECTED BY A LICENSED GEOTECHNICAL ENGINEER. WRITTEN FIELD REPORTS SHALL BE FORWARDED TO THE STRUCTURAL ENGINEER AS SOON AS THEY BECOME AVAILABLE.

FOUNDATION CONDITIONS NOTED DURING CONSTRUCTION. WHICH DIFFER FROM THOSE DESCRIBED IN THE GEOTECHNICAL REPORT, SHALL BE REPORTED TO THE STRUCTURAL ENGINEER AND GEOTECHNICAL ENGINEER BEFORE FURTHER CONSTRUCTION IS ATTEMPTED.

EXCAVATIONS FOR SPREAD FOOTINGS, COMBINED FOOTINGS, CONTINUOUS FOOTINGS AND MAT FOUNDATIONS SHALL BE CLEANED AND HAND TAMPED TO UNIFORM SURFACE AND SHALL BE PROTECTED AND MAINTAINED UNIFORM UNTIL CONCRETE IS PLACED.

ANY INSULATION SHOWN ON THE STRUCTURAL DRAWINGS IS FOR GENERAL VISUAL REFERENCE ONLY. UNLESS NOTED OTHERWISE. SEE THE ARCHITECTURAL DRAWINGS AND SPECIFICATIONS FOR EXACT LOCATION, PLACEMENT, THICKNESS AND MATERIAL REQUIREMENTS.

FLOWABLE FILL

WHERE INDICATED ON THE STRUCTURAL DRAWINGS, FLOWABLE FILL SHALL BE A CONTROLLED LOW-STRENGTH MATERIAL (CLSM) PRODUCED AND PLACED IN ACCORDANCE WITH RECOMMENDATIONS OF ACI 229. PROVIDE MATERIAL WITH 28-DAY COMPRESSIVE STRENGTH OF 1,000 PSI UNLESS NOTED OTHERWISE.

REINFORCED CONCRETE HAS BEEN DESIGNED IN ACCORDANCE WITH THE BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE (ACI 318) AND COMMENTARY

MIXING, TRANSPORTING, AND PLACING OF CONCRETE SHALL CONFORM TO THE LATEST EDITION OF THE SPECIFICATIONS FOR STRUCTURAL CONCRETE (ACI 301) READY-MIXED CONCRETE SHALL CONFORM TO THE REQUIREMENTS OF ASTM C94. IN CASE OF A DISCREPANCY, THE PLANS AND SPECIFICATIONS SHALL GOVERN.

CEMENT SHALL CONFORM TO ASTM C150, TYPE I, UNO.

FLY ASH SHALL CONFORM TO ASTM C618, CLASS C OR F. NORMAL WEIGHT AGGREGATES SHALL CONFORM TO ASTM C33.

WATER-REDUCING ADMIXTURES SHALL CONFORM TO ASTM C494.

AIR-ENTRAINING ADMIXTURES SHALL CONFORM TO ASTM C260 AND SHALL BE CERTIFIED BY THE MANUFACTURER TO BE COMPATIBLE WITH OTHER ADMIXTURES. CALCIUM CHLORIDE ADMIXTURES OR ADMIXTURES CONTAINING MORE THAN

0.1 PERCENT CHLORIDE IONS SHALL NOT BE USED. IN COLD WEATHER CONDITIONS, MIXING, PLACING, FINISHING, CURING AND PROTECTION OF CONCRETE SHALL BE PERFORMED IN ACCORDANCE WITH THE

LATEST EDITION OF ACI 306R, COLD WEATHER CONCRETING. IN HOT WEATHER CONDITIONS, MIXING, PLACING, FINISHING, CURING AND PROTECTION OF CONCRETE SHALL BE PERFORMED IN ACCORDANCE WITH THE LATEST EDITION OF ACI 305R, HOT WEATHER CONCRETING.

USE OF CONSTRUCTION JOINTS AT LOCATIONS OTHER THAN THOSE INDICATED ON THE DRAWINGS SHALL BE SUBMITTED TO THE STRUCTURAL ENGINEER FOR REVIEW. SLUMP FOR PUMPED CONCRETE SHALL BE MEASURED AT POINT OF DISCHARGE. NORMAL WEIGHT CONCRETE SHALL HAVE THE PROPERTIES AS INDICATED IN THE CONCRETE MIX SCHEDULE AND SPECIFICATIONS.

CONCRETE SLABS ON GRADE

ON THE SLAB SURFACE.

CURE SLABS ON GRADE AS FOLLOWS:

SLABS ON GRADE SHALL BE CONSTRUCTED IN ACCORDANCE WITH THE LATEST EDITION OF THE GUIDE FOR CONCRETE FLOOR AND SLAB CONSTRUCTION (ACI 302.1R). PLACE CONCRETE IN A MANNER SO AS TO PREVENT SEGREGATION OF THE MIX. DELAY FLOATING AND TROWELING OPERATIONS UNTIL THE CONCRETE HAS LOST SURFACE WATER SHEEN OR ALL FREE WATER. DO NOT SPRINKLE FREE CEMENT

PROVIDE CURING OF CONCRETE SLABS ON GRADE AS REQUIRED TO ACCOMMODATE FLOOR FINISHES AND FINISH MATERIALS PER THE ARCHITECTURAL DRAWINGS AND SPECIFICATIONS. REVIEW ALL FLOOR FINISH REQUIREMENTS PRIOR TO PLACEMENT OF CONCRETE SLABS AND COORDINATE SLAB MIX, PLACEMENT AND CURING TO COMPLY WITH FINISH FLOORING MATERIAL MANUFACTURER'S REQUIREMENTS.

CURING PROCEDURES SHALL COMPLY WITH ACI 302.1R AND USE OF CURING MATERIALS SHALL BE APPLIED IN STRICT ACCORDANCE WITH MANUFACTURER'S REQUIREMENTS. MOISTURE RETENTION COVERS, IF USED, SHALL BE APPLIED FLAT AND SMOOTH TO RESULT IN A UNIFORM APPEARANCE AND MINIMIZE SURFACE

MARKINGS AND BLEMISHES. UNLESS NOTED OTHERWISE, OR UNLESS MORE STRINGENT REQUIREMENTS APPLY,

SLABS RECEIVING FLOOR COVERING MATERIALS: MOIST CURE A MINIMUM OF 7 DAYS (OR, IF APPROVED BY FLOOR FINISH MATERIAL MANUFACTURER, USE ASTM C309, TYPE 1, CLASS B, WATERBORNE, MEMBRANE-FORMING CURING COMPOUND, DISSIPATING). REMOVAL OF CURING COMPOUND MATERIAL, IF REQUIRED FOR PROPER INSTALLATION OF FLOOR FINISH MATERIALS, SHALL BE PERFORMED BY THE CONCRETE CONTRACTOR.

EXPOSED SLABS UNO: ASTM C1315, TYPE 1, CLASS A, WATERBORNE, MEMBRANE-FORMING CURING AND SEALING COMPOUND.

PROVIDE SLABS ON GRADE WITH A SMOOTH TROWEL FINISH UNLESS NOTED

CONTRACTOR SHALL REVIEW ALL REQUIRED FLOOR FINISH MATERIAL REQUIREMENTS PRIOR TO PLACEMENT OF CONCRETE AND SHALL PROVIDE FLOOR SLAB FLATNESS AND LEVELNESS MEETING THE FINISH MATERIAL SUPPLIER'S WRITTEN REQUIREMENTS. FLATNESS AND LEVELNESS SHALL BE THE MORE STRINGENT OF THE FINISH MATERIAL REQUIREMENTS AND THE FOLLOWING:

SEE THE SLAB ON GRADE FLATNESS / LEVELNESS SCHEDULE.

CONTRACTOR SHALL PROVIDE ALL NECESSARY REPAIR, GRINDING AND/OR LEVELING OF THE CONCRETE SLAB TO ACCOMMODATE ALL FLOOR FINISHES PRIOR TO INSTALLATION OF THE FINISH MATERIALS WITH NO ADDITIONAL COST TO THE PROJECT.

THE MINIMUM LOCAL AREA SHALL BE ANY BAY DEFINED BY COLUMN LINES. UNLESS SHOWN OR NOTED OTHERWISE. PROVIDE CONTROL OR CONSTRUCTION JOINTS IN SLABS-ON-GRADE AT A MAXIMUM SPACING OF 36 TIMES THE SLAB THICKNESS. PROVIDE JOINTS AT ALL COLUMN LOCATIONS. LOCATE JOINTS TO ELIMINATE RE-ENTRANT CORNERS AND TO CREATE SQUARE OR RECTANGULAR SECTIONS WITH MAXIMUM LONG SIDE TO SHORT SIDE RATIO OF 1.5 TO 1.

CONTROL JOINTS IN SLABS ON GRADE SHALL NOT RECEIVE JOINT FILLER MATERIAL UNLESS NOTED OTHERWISE.

FINISH SLABS WITH FIBER REINFORCEMENT IN ACCORDANCE WITH THE FIBER MANUFACTURER'S RECOMMENDATIONS AND AS REQUIRED TO MINIMIZE FIBER EXPOSURE AT THE SLAB SURFACE. FOR SLABS WITH APPLIED FINISHES AND/OR MATERIALS, INCLUDING POLISHED CONCRETE FINISHES WHEN USED, REMOVE ALL PROTRUDING FIBERS PER THE FIBER MANUFACTURER'S RECOMMENDATIONS AND AS REQUIRED TO PROPERLY INSTALL THE FINISHES AND/OR MATERIALS. FOR SLABS EXPOSED TO VIEW IN THE COMPLETED CONSTRUCTION, REMOVE ALL PROTRUDING FIBERS PRIOR TO COMPLETION OF CONSTRUCTION PER THE FIBER MANUFACTURER'S RECOMMENDATIONS.

VAPOR RETARDER

VAPOR RETARDER SHALL BE ASTM E1745 CLASS A WITH A PERMEANCE OF 0.1 PERMS OR LESS, UNO. INSTALL, INSPECT AND REPAIR IN ACCORDANCE WITH ASTM E1643 AND MANUFACTURER'S WRITTEN REQUIREMENTS. INSTALL VAPOR RETARDER ONLY AT LOCATIONS SPECIFICALLY INDICATED.

REINFORCING STEEL

REINFORCING STEEL SHALL BE DEFORMED BARS CONFORMING TO ASTM A615 AND SHALL HAVE A MINIMUM YIELD STRENGTH OF 60,000 PSI. REINFORCING BAR DETAILING, FABRICATING, AND PLACING SHALL CONFORM TO THE

LATEST EDITION OF THE FOLLOWING STANDARDS: ACI 301, ACI 315, ACI 318, ACI DETAILING MANUAL (SP66), AND CRSI MANUAL OF STANDARD PRACTICE. THE CONTRACTOR SHALL PROVIDE BAR SUPPORTS AND SPACERS AS REQUIRED TO MAINTAIN PROPER SUPPORT AND POSITIONING OF THE REINFORCING STEEL THROUGHOUT CONCRETE PLACEMENT OPERATIONS. DESIGN OF THE SUPPORT

SYSTEM SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR. UNLESS A GREATER AMOUNT OF COVER IS INDICATED ON THE DRAWINGS. PROVIDE THE FOLLOWING MINIMUM CONCRETE COVER OVER REINFORCEMENT AS FOLLOWS:

CONCRETE CAST AGAINST EARTH: 3 INCHES FOUNDATION TOP REINFORCEMENT: 2 INCHES FOUNDATION BOTTOM AND SIDE REINFORCEMENT: 3 INCHES

REINFORCING STEEL SHALL NOT BE TACK WELDED, WELDED, HEATED OR CUT UNLESS INDICATED ON THE CONTRACT DOCUMENTS OR APPROVED BY THE STRUCTURAL ENGINEER.

WHERE LAP SPLICE LENGTHS ARE NOT SHOWN OR NOTED, PROVIDE A CLASS "B" LAP. ALL 90 DEGREE AND 180 DEGREE BENDS SHOWN ON THE DRAWINGS SHALL BE STANDARD HOOKS, UNLESS NOTED OTHERWISE.

PROVIDE CORNER BARS OF SAME SIZE AND SPACING AS HORIZONTAL BARS AT CORNERS OF ALL WALLS AND GRADE BEAMS. LAP SPLICE CORNERS BARS WITH

UNLESS OTHERWISE SHOWN OR NOTED, PROVIDE 2-#5 BARS (ONE EACH FACE) AROUND UNFRAMED OPENINGS IN CONCRETE WALLS AND GRADE BEAMS. PLACE BARS PARALLEL TO THE SIDES OF THE OPENING AND EXTEND 24" BEYOND CORNERS.

FIBER REINFORCEMENT (SYNTHETIC)

MICRO-FIBER REINFORCEMENT SHALL BE VIRGIN (NON-RECYCLED) NYLON OR POLYPROPYLENE FIBERS COMPLYING WITH ASTM C1116, WITH LONGEST FIBERS NOT

MACRO-FIBER REINFORCEMENT SHALL BE VIRGIN (NON-RECYCLED) POLYPROPYLENE / POLYETHYLENE FIBER BLEND COMPLYING WITH ASTM C1116, WITH LONGEST FIBERS NOT LESS THAN 1.75 INCHES.

FIBERS SHALL BE INTRODUCED INTO CONCRETE MIX AT THE BATCH PLANT AND MIXED IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS. DOSAGE RATES SHALL BE AS INDICATED IN THE CONTRACT DOCUMENTS. USE ONLY THE FIBER TYPE INDICATED FOR EACH APPLICATION. SUBSTITUTION OF FIBER TYPES IS NOT PERMITTED.

CONCRETE TESTING

MAKE ONE SET OF TEST CYLINDERS IN ACCORDANCE WITH ASTM C31 FOR EACH DAY'S POUR AND FOR EACH 100 CUBIC YARDS FOR EACH TYPE OF CONCRETE PLACED. EACH SET SHALL INCLUDE ONE SPECIMEN TESTED AT 7 DAYS, 2 SPECIMENS TESTED AT 28 DAYS (3 SPECIMENS TESTED AT 28 DAYS IF USING 4x8 CYLINDERS) AND ONE SPECIMEN RETAINED IN RESERVE TO BE TESTED AT THE DIRECTION OF THE STRUCTURAL ENGINEER. SPARE CYLINDER MAY BE DISCARDED 90 DAYS AFTER CASTING UNLESS DIRECTED OTHERWISE BY THE STRUCTURAL ENGINEER. THIS SET OF TEST CYLINDERS SHALL BE PROTECTED AGAINST FREEZING.

WHEN THE AMBIENT TEMPERATURE IS EXPECTED TO FALL BELOW 40 DEGREES DURING THE COURSE OF A CONCRETE POUR OR SUBSEQUENT CURING PROCESS, AN ADDITIONAL SET OF CONCRETE TEST CYLINDERS SHALL BE MADE AND TESTED. THESE CYLINDERS SHALL BE STORED IMMEDIATELY ADJACENT TO, AND CURED UNDER THE SAME CONDITIONS AS THE BUILDING CONCRETE. SPECIAL CURING BOXES ARE NOT PERMITTED FOR THESE TEST CYLINDERS.

FORWARD COPIES OF TEST RESULTS TO THE ARCHITECT. STRUCTURAL ENGINEER. READY-MIX SUPPLIER AND CONTRACTOR WITHIN 24 HOURS AFTER TESTING.

POTENTIALLY LOW-STRENGTH CONCRETE: CONCRETE WHICH EXHIBITS STRENGTH BASED ON 7-DAY TESTS BELOW 70% OF THE SPECIFIED 28-DAY COMPRESSIVE STRENGTH SHALL BE CONSIDERED DEFECTIVE WORK AND SHALL BE SUBJECT TO REPAIR OR REPLACEMENT BY THE CONTRACTOR AT THE DISCRETION OF THE OWNER AND STRUCTURAL ENGINEER AT NO ADDITIONAL COST TO THE OWNER.

CONCRETE UNIT MASONRY

CONCRETE MASONRY HAS BEEN DESIGNED IN ACCORDANCE WITH THE LATEST EDITION OF THE BUILDING CODE REQUIREMENTS FOR CONCRETE MASONRY STRUCTURES (ACI 530).

CONCRETE MASONRY SHALL CONSIST OF HOLLOW UNITS CONFORMING TO THE REQUIREMENTS OF ASTM C90, WITH A MINIMUM NET AREA COMPRESSIVE STRENGTH OF 2,800 PSI. CONCRETE MASONRY ASSEMBLAGES SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH (F'M) OF 2,000 PSI AT 28 DAYS.

ALL MASONRY TO BE LAID IN RUNNING BOND, UNO.

MORTAR SHALL BE TYPE S PROPORTIONED IN ACCORDANCE WITH ASTM C270. GROUT FOR REINFORCED MASONRY SHALL BE PROPORTIONED IN ACCORDANCE WITH ASTM C476. COARSE AND FINE AGGREGATES SHALL CONFORM TO ASTM C404. USE COARSE GROUT FOR ALL GROUTING EXCEPT HIGH-LIFT POURS DEFINED BY ACI 530 TABLE 1.16.1, WHERE FINE GROUT SHALL BE USED.

PROVIDE 9-GAUGE GALVANIZED STEEL WIRE JOINT REINFORCEMENT IN ALL MASONRY CONSTRUCTION. REINFORCEMENT SHALL BE CONTINUOUS AND BE LAPPED EIGHT INCHES AT SPLICES. CUT REINFORCEMENT AT ALL CONTROL AND EXPANSION JOINTS. SPACE REINFORCEMENT AT 8" ON CENTER FOR PARAPETS AND BELOW GROUND FLOOR ELEVATION. ELSEWHERE SPACE REINFORCEMENT AT 16 INCHES ON CENTER.

BEAMS AND LINTELS SHALL BEAR A MINIMUM OF 8 INCHES ONTO SUPPORTING MASONRY, UNLESS NOTED OTHERWISE. BEARING FOR ALL BEAMS, LINTELS, JOISTS. ETC. SHALL BE GROUTED SOLID A MINIMUM OF ONE COURSE (8 INCHES) BELOW BEARING ELEVATION, UNLESS NOTED OTHERWISE.

PROVIDE MASONRY CONTROL JOINTS AT 25'-0" OC MAXIMUM, UNO. LOCATE JOINTS IN ACCORDANCE WITH NCMA TEK 10-2C "CONTROL JOINTS FOR CONCRETE MASONRY WALLS - EMPIRICAL METHOD" AT OFFSETS, SETBACKS, WALL INTERSECTIONS, CORNERS, CHANGES IN THICKNESS, CHANGES IN HEIGHT, OR EDGE OF OPENINGS.

BRICK VENEER MASONRY

PROVIDE EXPANSION JOINTS IN ACCORDANCE WITH THE ARCHITECTURAL DRAWINGS AND SPECIFICATIONS.

IF BRICK EXPANSION JOINT LOCATIONS ARE NOT INDICATED ON THE ARCHITECTURAL DRAWINGS OR IN THE SPECIFICATIONS, SUBMIT PROPOSED EXPANSION JOINT LOCATIONS TO THE ARCHITECT AND STRUCTURAL ENGINEER FOR REVIEW PRIOR TO CONSTRUCTION. UNLESS INDICATED OTHERWISE. PROVIDE EXPANSION JOINTS AT 20'-0" OC MAXIMUM. LOCATE JOINTS IN ACCORDANCE WITH BIA TECHNICAL NOTE 18A "ACCOMMODATING EXPANSION OF BRICKWORK" AT OFFSETS, SETBACKS, WALL INTERSECTIONS, CORNERS, CHANGES IN THICKNESS, CHANGES IN HEIGHT, OR EDGES OF OPENINGS.

EXPANSION ANCHORS

EXPANSION ANCHORS SHALL BE HILTI "KWIK BOLT TZ" OR AN EQUIVALENT SUBSTITUTE APPROVED BY THE STRUCTURAL ENGINEER, UNO. ANCHOR FINISH SHALL BE CARBON STEEL FOR INTERIOR EXPOSURES AND

AISI 316 STAINLESS STEEL FOR EXTERIOR AND BELOW GRADE EXPOSURES, UNO. ANCHORS SHALL BE INSTALLED IN CONFORMANCE WITH THE MANUFACTURER'S REQUIREMENTS BY INSTALLERS TRAINED BY THE MANUFACTURER'S REPRESENTATIVE. ANCHORS SHALL NOT BE INSTALLED IN CONCRETE OR MASONRY UNTIL IT HAS ATTAINED ITS SPECIFIED MINIMUM 28 DAY COMPRESSIVE STRENGTH.

ADHESIVE ANCHORS FOR SOLID SUBSTRATES (THREADED ROD AND REBAR)

ADHESIVE ANCHORS FOR SOLID SUBSTRATES SHALL BE HILTI "HIT-HY 200" OR AN EQUIVALENT SUBSTITUTE APPROVED BY THE STRUCTURAL ENGINEER, UNO. THREADED ANCHORS SHALL BE ASTM A36 RODS FOR INTERIOR EXPOSURES AND AISI 316 STAINLESS STEEL FOR EXTERIOR AND BELOW GRADE EXPOSURES, UNO.

ANCHORS SHALL BE INSTALLED IN CONFORMANCE WITH THE MANUFACTURER'S REQUIREMENTS BY INSTALLERS TRAINED BY THE MANUFACTURER'S REPRESENTATIVE. ADHESIVE ANCHORS SHALL NOT BE INSTALLED IN CONCRETE OR MASONRY UNTIL IT HAS ATTAINED ITS SPECIFIED MINIMUM 28 DAY COMPRESSIVE STRENGTH. ADDITIONALLY, ADHESIVE ANCHORS SHALL NOT BE INSTALLED IN CONCRETE OR

MINIMUM EMBEDMENT DEPTH SHALL BE 8 DIAMETERS UNLESS NOTED OTHERWISE. ADHESIVE ANCHORS FOR HOLLOW SUBSTRATES

MASONRY WITH AN AGE OF LESS THAN 21 DAYS.

ADHESIVE ANCHORS FOR HOLLOW SUBSTRATES SHALL BE HILTI "HIT-HY 270" WITH HILTI "HIT-SC" SCREEN TUBE OR AN EQUIVALENT SUBSTITUTE APPROVED BY THE STRUCTURAL ENGINEER, UNO.

ANCHORS SHALL BE ASTM A36 THREADED RODS FOR INTERIOR EXPOSURES AND AISI 316 STAINLESS STEEL FOR EXTERIOR AND BELOW GRADE EXPOSURES, UNO. ANCHORS SHALL BE INSTALLED IN CONFORMANCE WITH THE MANUFACTURER'S REQUIREMENTS BY INSTALLERS TRAINED BY THE MANUFACTURER'S REPRESENTATIVE.

NON-SHRINK GROUT

GROUT SHALL BE A NON-METALLIC. SHRINKAGE RESISTANT (WHEN TESTED IN ACCORDANCE WITH THE LATEST EDITION OF ASTM C827 OR CRD-C621), PREMIXED, NON-CORROSIVE, NON-STAINING PRODUCT CONTAINING PORTLAND CEMENT, SILICA SANDS. SHRINKAGE COMPENSATING AGENTS AND FLUIDITY IMPROVING COMPOUNDS. GROUT SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH (F'C) OF 5,000 PSI IN

MASTIC COATING

MASTIC COATING FOR PROTECTION OF INDICATED ITEMS SHALL BE BITUMASTIC 50 COAL TAR MASTIC BY CARBOLINE OR EQUIVALENT SUBSTITUTE APPROVED BY THE STRUCTURAL ENGINEER. INSTALL AT LOCATIONS INDICATED ON DRAWINGS.

PROVIDE FULL COVERAGE OVER ITEMS INDICATED TO RECEIVE COATING. APPLY MASTIC COATING TO ALL STEEL ITEMS THAT WILL BE IN PERMANENT CONTACT WITH SOIL OR FILL MATERIALS IN THE COMPLETED CONSTRUCTION.

UNLESS NOTED OTHERWISE, APPLY MASTIC TO A COATING THICKNESS OF 18 MILS.

STRUCTURAL STEEL DETAILING, FABRICATION AND ERECTION SHALL CONFORM TO THE ANSI/AISC 360 SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS, LATEST EDITION WITH AMENDMENTS, AND THE AISC 303 CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES, LATEST EDITION WITH AMENDMENTS. STRUCTURAL STEEL WIDE FLANGE SHAPES SHALL CONFORM TO ASTM A992. STRUCTURAL STEEL PLATES AND ROLLED SHAPES OTHER THAN WIDE-FLANGE

SHAPES SHALL CONFORM TO ASTM A36, UNLESS NOTED OTHERWISE. HOLLOW STRUCTURAL STEEL SECTIONS (SQUARE, RECTANGULAR, AND ROUND TUBES) SHALL CONFORM TO ASTM A500 GRADE C, UNLESS NOTED OTHERWISE. STRUCTURAL PIPE SHALL CONFORM TO ASTM A53, GRADE B.

STRUCTURAL STEEL ROD HANGERS AND BRACING SHALL CONFORM TO ASTM A36, UNLESS NOTED OTHERWISE.

ANCHOR RODS SHALL CONFORM TO ASTM F1554, GRADE 36, UNLESS NOTED OTHERWISE.

BOLTS IN STRUCTURAL STEEL JOINTS SHALL CONFORM TO ASTM F3125 AND SHALL BE 3/4" DIAMETER GRADE A325, UNLESS NOTED OTHERWISE. (TWIST-OFF TENSION CONTROL BOLTS, IF USED, SHALL CONFORM TO ASTM F3125 AND SHALL BE 3/4" DIAMETER GRADE F1852, UNLESS NOTED OTHERWISE.)

BOLTED CONNECTIONS SHALL CONFORM TO THE SPECIFICATION FOR STRUCTURAL JOINTS USING HIGH STRENGTH BOLTS, APPROVED BY THE RESEARCH COUNCIL ON STRUCTURAL CONNECTIONS. UNLESS NOTED OTHERWISE, BOLTED CONNECTIONS SHALL BE TIGHTENED TO THE SNUG TIGHT CONDITION, EXCEPT BOLTED CONNECTIONS IN BRACE ELEMENTS ARE TO BE FULLY PRETENSIONED WITH CLASS A FAYING SURFACES. BURNING OF HOLES IN STRUCTURAL STEEL IS EXPLICITLY PROHIBITED UNLESS

WRITTEN PERMISSION IS GRANTED BY THE STRUCTURAL ENGINEER. DO NOT FLAME-

CUT HOLES OR ENLARGE HOLES BY BURNING. DRILL HOLES IN STRUCTURAL STEEL.

DO NOT ENLARGE UNFAIR HOLES IN MEMBERS BY BURNING OR BY USING DRIFT PINS

REAM HOLES THAT MUST BE ENLARGED TO ADMIT BOLTS. FIELD MODIFICATIONS TO HOLES IN STRUCTURAL STEEL ARE EXPLICITLY PROHIBITED UNLESS WRITTEN PERMISSION IS GRANTED BY THE STRUCTURAL ENGINEER. WELDING PROCEDURES SHALL CONFORM TO THE LATEST EDITION OF THE AMERICAN WELDING SOCIETY'S STRUCTURAL WELDING CODE FOR STEEL ANSI/AWS D1.1. WELDED CONNECTIONS USING ASTM A992 STEEL AS A BASE METAL SHALL BE MADE

E70XX ELECTRODES. STRUCTURAL STEEL SHALL BE PROVIDED WITH THE FOLLOWING SHOP FINISH UNLESS

WITH E70XX LOW HYDROGEN ELECTRODES. UNLESS OTHERWISE SHOWN OR NOTED

ON THE DRAWINGS, OTHER WELDED CONNECTIONS MAY BE MADE WITH STANDARD

STRUCTURAL STEEL THAT RECEIVES FINISH PAINT SHALL BE SHOP-PRIMED WITH RUST-INHIBITING PRIMER. VERIFY PRIMER IS COMPATIBLE WITH FINISH COAT SYSTEM SPECIFIED BY THE ARCHITECT. COORDINATE FINISH PAINTING REQUIREMENTS WITH THE ARCHITECT. STRUCTURAL STEEL THAT IS NOT EXPOSED IN THE FINISHED CONSTRUCTION AND DOES NOT RECEIVE FINISH PAINT SHALL NOT BE SHOP-PRIMED.

ALL EXTERIOR STRUCTURAL STEEL EXPOSED TO WEATHER SHALL BE HOT DIP GALVANIZED. UNLESS NOTED OTHERWISE.

STRUCTURAL STEEL NOTED TO BE GALVANIZED SHALL BE HOT-DIP GALVANIZED IN

CONFORMANCE WITH ASTM A123. DO NOT PAINT OR GALVANIZE THE FOLLOWING SURFACES:

2. SURFACES TO RECEIVE BOLTED SLIP-CRITICAL CONNECTIONS.

SPECIFICALLY NOTED OTHERWISE:

SURFACES TO BE WELDED

3. SURFACES TO RECEIVE SHEAR STUD CONNECTIONS. 4. SURFACES TO RECEIVE SPRAYED-ON FIREPROOFING. ALL ABRASIONS TO GALVANIZED SURFACES OR SURFACES TO RECEIVE AN

ARCHITECTURAL FINISH COAT SHALL BE TOUCHED-UP AFTER ERECTION IS COMPLETE. FOR PAINTED STEEL, USE A PRIMER EQUIVALENT TO THE SHOP PAINT. FOR GALVANIZED STEEL, USE A ZINC-RICH COLD-GALVANIZING PAINT. DESIGN CONNECTIONS NOT SHOWN IN ACCORDANCE WITH THE ASD SPECIFICATION AND MANUAL OF STEEL CONSTRUCTION. UNLESS NOTED OTHERWISE ON THE

DRAWINGS, DESIGN BEAM CONNECTIONS NOT SHOWN, TO SUPPORT A MINIMUM

SHEAR REACTION OF 16 KIPS ASD. PROVIDE NO LESS THAN 2 BOLTS IN ANY VERTICAL ROW OF BOLTS, UNLESS SPECIFICALLY INDICATED ON THE DRAWINGS. AT STRUCTURAL STEEL AND STEEL JOIST FRAMED ROOFS PROVIDE L4X4X1/4 FRAMED OPENINGS FOR ALL ROOF PENETRATIONS 12 INCHES OR LARGER ALONG ANY SIDE, UNLESS LARGER FRAMING IS INDICATED. REFER TO MECHANICAL AND PLUMBING DRAWINGS FOR PENETRATIONS. COORDINATE FRAMED OPENING SIZES AND

LOCATIONS WITH THE MECHANICAL AND PLUMBING CONTRACTORS. <u>DEFORMED BAR ANCHORS AND HEADED ANCHOR STUDS</u>

DEFORMED BAR ANCHORS (DBA) AND HEADED ANCHOR STUDS (HAS) USED TO ANCHOR STEEL ELEMENTS TO CONCRETE SHALL BE SUPPLIED AND INSTALLED BY THE STRUCTURAL STEEL CONTRACTOR, UNLESS NOTED OTHERWISE.

EQUIPMENT AND APPROPRIATE FERRULE IN ACCORDANCE WITH APPLICABLE PROVISIONS OF AWS D1.5. DEFORMED BAR ANCHORS SHALL BE TYPE C DEFORMED STEEL BARS MANUFACTURED IN ACCORDANCE WITH THE REQUIREMENTS OF ASTM A496, WITH A

ANCHORS SHALL BE FULL-BASE WELDED USING AUTOMATICALLY TIMED WELDING

YIELD STRENGTH OF 70 KSI. HEADED ANCHOR STUDS SHALL BE MANUFACTURED IN ACCORDANCE WITH THE

REQUIREMENTS OF ASTM A108, WITH A YIELD STRENGTH OF 50 KSI.

SEPARATION OF DISSIMILAR METALS

DISSIMILAR METALS INCLUDING STRUCTURAL SHAPES, PLATES, CONNECTORS, ETC. SHALL BE ELECTRICALLY ISOLATED TO PREVENT GALVANIC CORROSION VIA NON-CONDUCTIVE WASHERS, SHOULDER WASHERS, GASKETS, COATINGS, OR EQUIVALENT SUBSTITUTE APPROVED BY THE STRUCTURAL ENGINEER, UNO.

STEEL JOISTS

STEEL JOISTS SHALL BE DESIGNED, FABRICATED, ERECTED AND BRACED IN ACCORDANCE WITH THE LATEST STEEL JOIST INSTITUTE (SJI) SPECIFICATIONS.

SPECIAL JOISTS SHALL BE FABRICATED IN ACCORDANCE WITH THE DESIGNATION SHOWN FOR EACH JOIST ON THE DRAWINGS. THE DESIGNATIONS FOLLOW THE FORM:

WHERE DD INDICATES THE MAXIMUM JOIST DEPTH, K INDICATES THE JOIST SERIES. TL/LL INDICATES THE DESIGN TOTAL LOAD AND LIVE LOAD RESPECTIVELY IN POUNDS PER LINEAL FOOT.

DEFLECTION OF STEEL JOISTS SHALL BE LIMITED TO THE FOLLOWING: LL DEFLECTION DL+LL DEFLECTION

SHOWN OR NOTED, PROVIDE ANCHORAGE AS REQUIRED BY THE SJI SPECIFICATIONS. HORIZONTAL BRIDGING AND DIAGONAL BRIDGING FOR STEEL JOISTS SHALL BE LOCATED AND DESIGNED AS REQUIRED BY THE SJI SPECIFICATIONS. BRIDGING MEMBERS SHALL BE CONNECTED TO THE JOIST CHORDS BY WELDING OR OTHER MECHANICAL MEANS. THE ENDS OF BRIDGING LINES TERMINATING AT CONCRETE WALLS, CONCRETE BLOCK WALLS OR STEEL BEAMS SHALL BE SECURELY ANCHORED THERETO AT TOP AND BOTTOM CHORDS. BRIDGING SHALL RUN THE FULL LENGTH/ WIDTH OF THE BUILDING.

WHERE BEARING DEPTH, LENGTH AND END ANCHORAGE FOR STEEL JOISTS ARE NOT

HANGERS AND OTHER SUPPORTS SHALL BE LOCATED AT THE INTERSECTION OF THE CHORD AND WEB MEMBERS (PANEL POINTS). CONCENTRATED LOADS IN EXCESS OF 100 POUNDS OTHER THAN THOSE SHOWN ON THE DRAWINGS SHALL NOT BE APPLIED WITHOUT PRIOR APPROVAL OF THE STRUCTURAL ENGINEER.

WHERE COLUMNS ARE NOT FRAMED IN AT LEAST TWO DIRECTIONS WITH STEEL BEAMS THE JOIST AT OR NEAREST EACH COLUMN LINE SHALL BE BOLTED TO

SUPPORTING MEMBERS DURING ERECTION.

BETWEEN COLUMN FLANGES.

STEEL JOISTS SHALL BE SUPPLIED WITH A SHOP COAT OF RUST-INHIBITIVE PRIMER PAINT. COORDINATE PRIMER PAINT WITH THE ARCHITECT FOR JOISTS TO REMAIN EXPOSED OR TO RECEIVE FINISH PAINT.

JOIST MANUFACTURER SHALL TRIM JOIST SEATS IN THE SHOP AS REQUIRED TO FIT

STEEL JOIST GIRDERS

STEEL JOIST GIRDERS SHALL BE DESIGNED. FABRICATED. ERECTED AND BRACED IN ACCORDANCE WITH THE LATEST STEEL JOIST INSTITUTE (SJI) SPECIFICATIONS. JOIST GIRDERS THAT ARE PART OF A MOMENT RESISTIVE RIGID FRAME SHALL BE DESIGNED FOR THE INDICATED END MOMENTS.

JOIST GIRDERS SHALL BE FABRICATED IN ACCORDANCE WITH THE DESIGN SPECIFICATION SHOWN FOR EACH JOIST GIRDER ON THE DRAWINGS. THE DESIGN SPECIFICATIONS FOLLOW THE FORM:

DD G XN TLK/LLK

WHERE DD INDICATES THE MAXIMUM JOIST GIRDER DEPTH, G INDICATES JOIST GIRDER TYPE. X INDICATES THE NUMBER OF JOIST SPACES (EQUAL OR UNEQUAL) OCCURRING IN THE JOIST GIRDER SPAN, AND TLK/LLK INDICATES THE TOTAL AND LIVE PANEL POINT DESIGN LOAD RESPECTIVELY (EXCLUDING JOIST GIRDER SELF WEIGHT) IN KIPS. WHERE "VG" REPLACES "G", JOISTS MUST BEAR ONLY AT VERTICAL JOIST GIRDER WEB MEMBERS.

DEFLECTION OF STEEL JOIST GIRDERS SHALL BE LIMITED TO THE FOLLOWING:

LL DEFLECTION DL+LL DEFLECTION ROOF JOIST GIRDERS:

STABILIZER PLATES SHALL BE PROVIDED ON THE COLUMNS BETWEEN THE JOIST GIRDER BOTTOM CHORD ANGLES AS REQUIRED BY S.J.I. SPECIFICATIONS. DO NOT WELD BOTTOM CHORD ANGLES TO STABILIZER UNLESS SPECIFICALLY SHOWN OR NOTED ON THE DRAWINGS.

WHERE END ANCHORAGE FOR JOIST GIRDERS ARE NOT SHOWN OR NOTED, PROVIDE ANCHORAGE AS REQUIRED BY THE SJI SPECIFICATIONS. JOIST GIRDER BRACING SHALL BE AS REQUIRED BY THE JOIST GIRDER

STEEL JOIST GIRDERS SHALL BE SUPPLIED WITH A SHOP COAT OF RUST-INHIBITIVE PRIMER PAINT. COORDINATE PRIMER PAINT WITH THE ARCHITECT FOR JOIST GIRDERS TO REMAIN EXPOSED OR TO RECEIVE FINISH PAINT. JOIST GIRDER MANUFACTURER SHALL TRIM JOIST SEATS IN THE SHOP AS REQUIRED

TO FIT BETWEEN COLUMN FLANGES.

STEEL ROOF DECK

MANUFACTURER.

PROVIDE AND ERECT STEEL DECK IN ACCORDANCE WITH THE LATEST EDITION OF THE STEEL DECK INSTITUTE'S SPECIFICATIONS AND CODE OF STANDARD PRACTICE. DECK MANUFACTURER SHALL PROVIDE ALL ROOF DECK ACCESSORIES, INCLUDING CLOSURES, SUPPLEMENTARY FRAMING, AND SUMP PANS, WHETHER OR NOT SUCH

FASTEN ROOF DECK TO STEEL SUPPORTS AS INDICATED ON THE DRAWINGS. PERFORM WELDING IN ACCORDANCE WITH ANSI/AWS D1.3.

PROVIDE STEEL ROOF DECK WITH FINISH AS INDICATED ON THE DRAWINGS.

ITEMS ARE DETAILED ON THE CONTRACT DOCUMENTS.

STEEL FRAMING DESIGNER PRIOR TO FABRICATION.

MAXIMUM).

ROOF DECK SHALL BE INSTALLED IN A MINIMUM THREE SPAN CONDITION WHEREVER POSSIBLE. WHERE THREE SPAN CONDITION IS NOT POSSIBLE, NOTIFY STRUCTURAL ENGINEER PRIOR TO FABRICATION OF DECK SO THAT EVALUATION OF THE LESSER SPAN CONDITION(S) CAN BE PERFORMED.

GLAZING SYSTEMS (CURTAIN WALL AND STOREFRONT) SUPPLIER OF GLAZING SYSTEM IS RESPONSIBLE FOR THE DESIGN, DETAILING AND INSTALLATION OF THE CURTAIN WALL SYSTEM.

FRAMING AND CONNECTIONS REQUIRED TO TRANSMIT THE GRAVITY AND/OR LATERAL LOADS TO THE STRUCTURE NOT DETAILED ON THE STRUCTURAL DRAWINGS IS THE RESPONSIBILITY OF THE GLAZING SYSTEM SUPPLIER TO DESIGN, DETAIL AND INSTALL CONNECTIONS TO TRANSMIT LATERAL LOADS TO THE STRUCTURE SHALL NOT BE MADE TO THE BOTTOM FLANGES OF BEAMS, BOTTOM CHORDS OF JOIST GIRDERS OR

BOTTOM CHORDS OF JOISTS WITHOUT APPROVAL OF THE STRUCTURAL ENGINEER.

WHERE COLD-FORMED STEEL FRAMING KNEE WALLS ARE PRESENT UNDER THE GLAZING SYSTEM THE VERTICAL MULLIONS SHALL BE SUPPORTED BY STRUCTURAL FRAMING (WHERE INDICATED). WHERE STRUCTURAL FRAMING IS NOT INDICATED, VERTICAL MULLIONS SHALL BE SUPPORTED BY COLD-FORMED STEEL KNEE WALL FRAMING DESIGNED BY THE COLD-FORMED STEEL DESIGNER FOR THE APPROPRIATE REACTIONS PROVIDED BY THE GLAZING SYSTEM DESIGNER. COORDINATE LOCATIONS, REACTIONS, AND CONNECTION REQUIREMENTS WITH THE COLD-FORMED

COLD-FORMED STEEL FRAMING (STUDS, TRACK, ETC.) THE SUPPLIER OF THE COLD-FORMED STEEL FRAMING SHALL BE RESPONSIBLE FOR THE DESIGN OF ALL COLD-FORMED STEEL MEMBERS, COMPONENTS, ASSEMBLIES AND

ACCORDANCE WITH REQUIREMENTS OF THE CURRENT AMERICAN IRON AND STEEL INSTITUTE (AISI) SPECIFICATIONS FOR THE DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS. ALL COLD-FORMED STEEL FRAMING MATERIALS SHALL CONFORM TO ASTM A653 WITH ZINC COATING CONFORMING TO ASTM A924. PLATES, GUSSETS AND CLIPS SHALL

HAVE A MINIMUM YIELD STRENGTH OF 40 KSI AND SHALL HAVE THICKNESS AS

THE MANUFACTURER'S PRINTED INSTRUCTIONS AND AS REVIEWED BY THE

STRUCTURAL PROPERTIES OF FRAMING MEMBERS SHALL BE CALCULATED IN

SHALL HAVE A MINIMUM YIELD STRENGTH OF 33 KSI. ALL FASTENERS SHALL CONFORM TO ASTM A90. ALL SCREWS SHALL BE SELF-TAPPING SELF-DRILLING.

REQUIRED FOR SPECIFIC CONDITIONS. TRACKS, BRACING, FURRING AND BRIDGING

STRUCTURAL ENGINEER. COLD-FORMED STEEL FRAMING SYSTEM SHALL BE INSTALLED TO ACCOMMODATE CONSTRUCTION TOLERANCES, DEFLECTION OF BUILDING STRUCTURAL MEMBERS, AND CLEARANCES OF INTENDED OPENINGS. PROVIDE DEFLECTION ALLOWANCE IN STUD TRACK, DIRECTLY BELOW BUILDING FRAMING FOR NON-LOAD BEARING FRAMING.

INSTALL WALL STUD BRIDGING PER THE MANUFACTURER'S REQUIREMENTS (4'-0" O.C.

COLD-FORMED STEEL FRAMING SYSTEM SHALL BE INSTALLED IN ACCORDANCE WITH

DESIGN DATA

SNOW LOAD DESIGN DATA

WIND LOAD DESIGN DATA

WIND EXPOSURE = C

EARTHQUAKE DESIGN DATA

RISK CATEGORY = II

SITE CLASSIFICATION = D

LATERAL LOAD RESISTING SYSTEM

PLAN NORTH-SOUTH DIRECTION:

STEEL BRACED FRAMES

PLAN EAST-WEST DIRECTION:

STEEL MOMENT FRAMES

SEISMIC DESIGN CATEGORY = B

GROUND SNOW LOAD $(P_G) = 30 PSF$

BASIC WIND SPEED (3-SECOND GUST) = 115 MPH

SHORT PERIOD SPECTRAL RESPONSE $(S_S) = 0.121$

SHORT PERIOD SPECTRAL RESPONSE (S_{DS}) = 0.129

RESPONSE MODIFICATION COEFFICIENT (R) = 3.0

SEISMIC RESPONSE COEFFICIENT (C_S) = 0.043

OF THE FOLLOWING, UNLESS NOTED OTHERWISE:

BASIC STRUCTURAL SYSTEM / SEISMIC RESISTING SYSTEM:

STEEL NOT SPECIFICALLY DETAILED FOR SEISMIC RESISTANCE

LATERAL FORCE RESISTANCE IN THE COMPLETED STRUCTURE IS PROVIDED AS

TRANSFER LATERAL LOADS TO THE VERTICAL LATERAL LOAD RESISTING ELEMENTS WHICH IN TURN TRANSFER THOSE LOADS TO THE FOUNDATION SYSTEM. VERTICAL

LATERAL LOAD RESISTING ELEMENTS ARE INDICATED ON THE DRAWINGS AND CONSIST

FOLLOWS: ELEVATED ROOF DECKS SERVE AS HORIZONTAL DIAPHRAGMS TO

ANALYSIS PROCEDURE UTILIZED = EQUIVALENT LATERAL FORCE PROCEDURE

1-SECOND SPECTRAL RESPONSE (S_{D1}) = 0.101

1-SECOND SPECTRAL RESPONSE $(S_1) = 0.063$

SEISMIC IMPORTANCE FACTOR (I_E) = 1.00

THERMAL FACTOR (C_T) = 1.0

NEW STRUCTURES ON THESE DRAWINGS HAVE BEEN DESIGNED IN ACCORDANCE WITH THE 2014 INDIANA BUILDING CODE.

SEE LOAD MAPS FOR DESIGN LOADS.

WIND DESIGN PRESSURE (P) = VARIES WITH HEIGHT AND LOCATION

SNOW EXPOSURE FACTOR (C_E) = 1.0 SNOW LOAD IMPORTANCE FACTOR (I_s) = 1.0 LOW-SLOPE ROOF SNOW LOAD (PF) = 21 PSF 7260 Shadeland Station MINIMUM LOW-SLOPE ROOF SNOW LOAD $(P_M) = 20 \text{ PSF}$ Indianapolis, IN 46256

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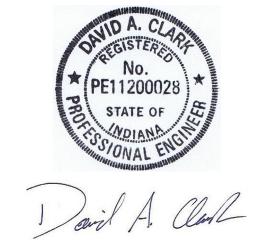
STRUCTUREPOIN

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PORTER COUNTY -

TRUSTEES OFFICE

PORTAGE, IN



ISSUANCE INDEX

08.20.18

PROJECT PHASE:

REVISION SCHEDULE

NO. DESCRIPTION DATE

100% CONSTRUCTION DOCUMENTS - BP1

Project Number 2017.01279

GENERAL NOTES

SPECIAL INSPECTION SERVICES SCHEDULE - STEEL CONSTRUCTION REFER TO IBC 2012 CHAPTER 17 AND AISC 360-10 APPLICABLE TO PERFORM OR ITEM TASK REFERENCE PROJECT (Y/N) OBSERVE WELDED MEMBERS **PRIOR TO WELDING:** WELDING PROCEDURE VERIFY CONTRACTOR MAINTAINS WELDING PROCEDURE SPECIFICATIONS PERFORM AISC 360 TABLE N5.4-1 SPECIFICATIONS READILY AVAILABLE FOR ALL WELDERS FOR ALL TYPES OF WELDS PERFORMED ON THE PROJECT WELD MATERIAL AISC 360 TABLE N5.4-7 VERIFY MATERIAL CERTIFICATIONS ARE AVAILABLE FOR ALL CONSUMABLE PERFORM CERTIFICATIONS WELDING MATERIALS WELD MATERIAL VERIFY ALL WELD MATERIALS ARE PROPERLY MARKED WITH VALID OBSERVE | AISC 360 TABLE N5.4-1 IDENTIFICATION TYPE/GRADE IDENTIFICATION VERIFY FABRICATOR/ERECTOR AS APPLICABLE MAINTAINS RECORDS OF WELDER IDENTIFICATION OBSERVE | AISC 360 TABLE N5.4-1 WHO WELDED EVERY JOINT FIT-UP OF GROOVE WELDS AISC 360 TABLE N5.4-1 INSPECT FOR PROPER FIT-UP INCLUDING JOINT PREPARATION. OBSERVE DIMENSIONS, CLEANLINESS, TACKING, AND BACKING (WHERE BACKING IS ACCESS HOLES INSPECT FOR CONFIGURATION AND FINISH OF ACCESS HOLES OBSERVE | AISC 360 TABLE N5.4-7 FIT-UP OF FILLET WELDS AISC 360 TABLE N5.4-INSPECT FOR PROPER FIT-UP INCLUDING DIMENSIONS, CLEANLINESS, AND OBSERVE TACKING **DURING WELDING:** QUALIFIED WELDERS | AISC 360 TABLE N5.4-2 VERIFY USE OF QUALIFIED WELDERS OBSERVE WELDING CONSUMABLES INSPECT FOR PROPER PACKAGING, STORAGE AND PROTECTION OBSERVE AISC 360 TABLE N5.4-2 TACK WELDS VERIFY NO WELDING OCCURS OVER CRACKED TACK WELDS OBSERVE | AISC 360 TABLE N5.4-2 FIELD CONDITIONS AISC 360 TABLE N5.4-2 VERIFY WIND SPEED WITHIN LIMITS AND PROPER OBSERVE PROTECTION/PREPARATION FOR PRECIPITATION AND TEMPERATURE WELDING PROCEDURE VERIFY WELDING PROCEDURE SPECIFICATIONS ARE FOLLOWED OBSERVE | AISC 360 TABLE N5.4-2 SPECIFICATION (NOTE 2 - SEE AISC 360) WELDING TECHNIQUES VERIFY EACH PASS MEETS PROFILE LIMITATIONS AND QUALITY | AISC 360 TABLE N5.4-2 OBSERVE REQUIREMENTS, INTERPASS AND FINAL CLEANING INSPECT FOR PROPER PLACEMENT AND INSTALLATION OF STEEL HEADED COMPOSITE BEAM HEADED OBSERVE STUD ANCHORS **AFTER WELDING:** WELDS CLEANED VERIFY PROPER CLEANING OF COMPLETED WELDS OBSERVE | AISC 360 TABLE N5.4-3 AISC 360 TABLE N5.4-3 SIZE, LENGTH, LOCATION, AND | INSPECT WELDS TO CONFIRM PROPER SIZE, LENGTH, LOCATION AND VISUAL ACCEPTANCE CRITERIA \parallel VISUAL ACCEPTANCE CRITERIA (CRACK PROHIBITION, WELD/BASE METAL FUSION, CRATER CROSS SECTION, WELD PROFILES, WELD SIZE, UNDERCUT (NOTE 3 - SEE AISC 360) POROSITY) AS FOLLOWS: VISUALLY INSPECT AND VERIFY ULTRASONIC TESTING IS PERFORMED FOR PERFORM 100% OF ALL CJP WELDS VISUALLY INSPECT 100% OF ALL CJP WELDS PERFORM VISUALLY INSPECT 100% OF WELDS IN LATERAL LOAD RESISTING MOMENT PERFORM FRAMES AND BRACED FRAMES VISUALLY INSPECT 20% OF ALL FILLET WELDS NOT IN LATERAL LOAD PERFORM RESISTING MOMENT FRAMES AND BRACED FRAMES ARC STRIKES PERFORM | AISC 360 TABLE N5.4-3 VISUALLY INSPECT TO CONFIRM NO ARC STRIKES "k"-AREA WELDING VISUALLY INSPECT ALL DOUBLER PLATES, CONTINUITY PLATES AND PERFORM | AISC 360 TABLE N5.4-3 STIFFENER PLATES WHERE WELDING OCCURS WITHIN THE k-AREA OF THE STEEL SECTION TO CONFIRM NO CRACKS IN THE MEMBER WEB BACKING AND WELD TABS VISUALLY INSPECT TO CONFIRM PROPER REMOVAL OF ALL BACKING AISC 360 TABLE N5.4-3 PERFORM REMOVED. VERIFY ALL WELD TABS REMOVED (IF REQUIRED) REMOVED VERIFY APPROVED REPAIRS OF NON-CONFORMING ITEMS (WHEN REPAIRS | AISC 360 TABLE N5.4-3 PERFORM REQUIRED) ARE PROPERLY COMPLETED AISC 360 TABLE N5.4-3 **DOCUMENTATION** DOCUMENT ACCEPTANCE OR REJECTION OF WELDED CONNECTIONS PERFORM **BOLTED MEMBERS** PRIOR TO BOLTING: MATERIAL CERTIFICATIONS VERIFY MANUFACTURER'S CERTIFICATIONS ARE AVAILABLE FOR FASTENER PERFORM AISC 360 TABLE N5.6-**FASTENER MARKINGS** VERIFY FASTENERS ARE MARKED PER ASTM STANDARDS OBSERVE | AISC 360 TABLE N5.6-7 FASTENER SELECTIONS VERIFY CORRECT GRADE, TYPE, AND SUFFICIENT LENGTH FOR THREADS OBSERVE | AISC 360 TABLE N5.6-1 EXCLUDED FROM THE SHEAR PLANE (ONLY WHERE SPECIFICALLY **BOLTING PROCEDURE** VERIFY PROPER BOLTING PROCEDURE FOR THE JOINT AISC 360 TABLE N5.6-OBSERVE CONNECTING ELEMENTS VERIFY CONNECTING ELEMENTS, INCLUDING FAYING SURFACES AND HOLE OBSERVE AISC 360 TABLE N5.6-PREPARATION (IF SPECIFIED) MEET APPLICABLE REQUIREMENTS PRE-INSTALLATION VERIFY PRE-INSTALLATION VERIFICATION TESTING BY INSTALLATION OBSERVE AISC 360 TABLE N5.6-VERIFICATION PERSONNEL IS OBSERVED AND DOCUMENTED FOR FASTENER ASSEMBLIES AND METHODS USED VERIFY PROPER STORAGE OF BOLTS, NUTS, WASHERS AND OTHER PROPER STORAGE OBSERVE AISC 360 TABLE N5.6-7 **FASTENER COMPONENTS DURING BOLTING:** VERIFY ALL FASTENING ELEMENTS ARE PROPERLY POSITIONED IN THE PROPER POSITIONING OBSERVE | AISC 360 TABLE N5.6-2 JOINT PRIOR TO TIGHTENING **BOLT TIGHTENING** VERIFY COMPONENT NOT TURNED BY THE WRENCH IS PREVENTED FROM OBSERVE AISC 360 TABLE N5.6-2 ROTATING DURING TIGHTENING PRETENSIONED BOLTS SNUG VERIFY ALL BOLTS AT A CONNECTION TO BE PRETENSIONED ARE FIRST OBSERVE AISC 360 TABLE N5.6-2 TIGHTENED BROUGHT INTO SNUG-TIGHT CONDITION PRIOR TO START OF PRE-PRETENSIONED BOLTS VERIFY FASTENERS ARE PRETENSIONED IN ACCORDANCE WITH RCSC | AISC 360 TABLE N5.6-2 SPECIFICATION FOR STRUCTURAL JOINTS USING HIGH-STRENGTH BOLTS **AFTER BOLTING:** DOCUMENTATION DOCUMENT ACCEPTANCE OR REJECTION OF BOLTED CONNECTIONS PERFORM | AISC 360 TABLE N5.6-3 STEEL FRAMING (GENERAL) **COLUMN BASES** INSPECT INSTALLATION OF ALL BASE PLATES FOR PROPER ANCHORAGE PERFORM (INCLUDING WELDED PLATE WASHERS WHERE INDICATED) AND GROUTING BENEATH BASE PLATE **COLUMN SPLICES** INSPECT ALL COLUMN SPLICES TO CONFIRM THAT GAPS BETWEEN PERFORM BEARING ELEMENTS DO NOT EXCEED 1/16-INCH BEAM, GIRDER AND COLUMN INSPECT FOR CONFORMANCE WITH REQUIRED SIZE, SPACING AND PERFORM MEMBERS CONNECTION REQUIREMENTS ROOF AND FLOOR DECK/SLAB | INSPECT SUPPORT MATERIALS FOR PROPER SIZE, POSITIONING AND PERFORM EDGE SUPPORTS CONNECTIONS BRACED FRAMES AND INSPECT INSTALLATION OF ALL MEMBERS AND CONNECTIONS IN BRACED PERFORM MOMENT FRAMES FRAMES AND MOMENT FRAMES FOR PROPER SIZE, POSITIONING AND CONNECTIONS COMPOSITE STEEL FRAMING INSPECT COMPOSITE STEEL FRAMING PRIOR TO PLACEMENT OF CONCRETE AS FOLLOWS: VERIFY PROPER PLACEMENT AND INSTALLATION OF STEEL DECK PERFORM | AISC 360 TABLE N6.1 INSPECT DIAMETER, LENGTH, QUANTITY AND POSITIONING OF STEEL PERFORM | AISC 360 TABLE N6.1 HEADED STUD ANCHORS RING TEST 100% OF HEADED STUD ANCHORS WITH A 3 LB HAMMER PERFORM BEND TEST QUESTIONABLE HEADED STUD ANCHORS TO 15 DEGREES PERFORM DOCUMENT ACCEPTANCE OR REJECTION OF COMPOSITE STEEL FRAMING PERFORM ELEMENTS STEEL GRATING INSPECT STEEL GRATING AS FOLLOWS: VFRIFY PROPER SIZE. TYPE AND FINISH PERFORM VERIFY PROPER ATTACHMENTS TO SUPPORTING STRUCTURE(S) PERFORM SEE CONTRACT DOCUMENTS FOR WELDING REQUIREMENTS. SEE AISC 360 FOR ADDITIONAL INFORMATION. SEE AISC 360 FOR SPECIFIC REQUIRED ELEMENTS OF WELD PROCEDURE SPECIFICATION.

	REFER TO IBC 2012 CHAPTER 17 TABLE 1705.6							
ITEM	TASK	APPLICABLE TO PROJECT (Y/N)	FREQUENCY	REFERENCE				
BEARING SOILS FOR SHALLOW FOUNDATIONS	VERIFY MATERIALS BELOW SHALLOW FOUNDATIONS ARE ADEQUATE TO ACHIEVE THE DESIGN BEARING CAPACITY	Y	PERIODIC					
EXCAVATIONS	VERIFY EXCAVATIONS ARE EXTENDED TO PROPER DEPTH AND HAVE REACHED PROPER MATERIAL	Y	PERIODIC					
FILL MATERIALS	VERIFY CLASSIFICATION AND TESTING OF COMPACTED FILL MATERIALS IS PERFORMED	Y	PERIODIC					
FILL MATERIAL PLACEMENT	VERIFY USE OF PROPER MATERIALS, DENSITIES AND LIFT THICKNESSES DURING PLACEMENT AND COMPACTION OF COMPACTED FILL	Y	CONTINUOUS					
SUBGRADE PREPARATION	PRIOR TO PLACEMENT OF COMPACTED FILL, INSPECT SUBGRADE AND VERIFY THAT SITE HAS BEEN PREPARED PROPERLY	Y	PERIODIC					
BACKFILL AT BELOW-GRADE WALLS	VERIFY THAT BELOW-GRADE WALLS WITH UNEVEN BACKFILL CONDITIONS ARE NOT BACKFILLED UNTIL FLOOR CONSTRUCTION AT TOPS OF WALLS (OR OTHER PERMANENT BRACING WHERE APPLICABLE) IS COMPLETE OR TEMPORARY BRACING IS PROVIDED	Y	PERIODIC					

"OBSERVE" REQUIRES OBSERVATION ON A RANDOM BASIS. OPERATIONS NEED NOT BE DELAYED PENDING THESE INSPECTIONS.

"PERFORM" REQUIRES TASKS TO BE PERFORMED FOR EACH OPERATION, JOINT OR MEMBER AS APPLICABLE.

SEE AISC 360 FOR SPECIFIC REQUIRED VISUAL ACCEPTANCE CRITERIA.

SPECIAL INSPECTIONS BASIC SERVICES - CONCRETE CONSTRUCTION

	REFER TO IBC 2012 CHAPTER 17 TABLE 1705.3 AND	ACI 318-11		
ITEM	TASK	APPLICABLE TO PROJECT (Y/N)	FREQUENCY	REFERENCE
REINFORCEMENT	INSPECT REINFORCEMENT, INCLUDING PRESTRESSING TENDONS (WHERE APPLICABLE) AND VERIFY CORRECT PLACEMENT	Y	PERIODIC	ACI 318 SECTIONS 3.5 AND 7.1 THROUGH 7.7; IBC 1910.4
REINFORCING BAR WELDING	VERIFY WELDABILITY OF BARS OTHER THAN ASTM A706	Y	PERIODIC	AWS D1.4, ACI 318 SECTION 3.5.2
ANCHORS AND EMBEDDED ITEMS CAST IN CONCRETE	INSPECT ALL ANCHORS AND EMBEDDED ITEMS FOR PROPER SIZE, TYPE, QUANTITY, LOCATION, POSITION, PROJECTION AND EMBEDMENT	Y	PERIODIC	ACI 318 SECTIONS 3.8.6, 8.1.3, 21.2.8; IBC 1908.5, 1909.1
ANCHORS POST-INSTALLED IN HARDENED CONCRETE	INSPECT ANCHORS POST-INSTALLED IN HARDENED CONCRETE MEMBERS	Y	PERIODIC	ACI 318 SECTIONS 3.8.6, 8.1.3, 21.2.8; IBC \1909.1
CONCRETE MIX	VERIFY USE OF REQUIRED CONCRETE MIX AT EACH APPLICATION	Y	PERIODIC	ACI 318 CHAPTER 4, SECTIONS 5.2 THROUGH 5.4; IBC 1904.2, 1910.2, 1910.3
CONCRETE TESTING	PERFORM CONCRETE SAMPLING AND TESTING IN ACCORDANCE WITH CONSTRUCTION DOCUMENTS	Y	CONTINUOUS	ASTM C172, ASTM C31, ACI 318 5.6, 5.8; IBC 1910.10
CONCRETE PLACEMENT	INSPECT FOR PROPER PLACEMENT TECHNIQUES	Y	CONTINUOUS	ACI 318 SECTIONS 5.9, 5.10; IBC 1910.6, 1910.7, 1910.8
SHOTCRETE PLACEMENT	INSPECT FOR PROPER APPLICATION TECHNIQUES	N	CONTINUOUS	ACI 318 SECTIONS 5.9, 5.10; IBC 1910.6, 1910.7, 1910.8
CONCRETE CURING	VERIFY MAINTENANCE OF PROPER CONCRETE TEMPERATURE AND CURING TECHNIQUES	Y	PERIODIC	ACI 318 SECTIONS 5.11 THROUGH 5.13; IBC 1910.9
PRESTRESSED CONCRETE	INSPECT FOR PROPER APPLICATION OF PRESTRESSING FORCES	N	CONTINUOUS	ACI 318 SECTION 18.2
PRESTRESSED CONCRETE	INSPECT FOR PROPER GROUTING OF BONDED PRESTRESSING TENDONS	N	CONTINUOUS	ACI 318 SECTION 18.18.4
ERECTION OF PRECAST CONCRETE MEMBERS	VERIFY WELDING PROCEDURE SPECIFICATIONS ARE FOLLOWED	N	PERIODIC	ACI CHAPTER 16
ERECTION OF PRECAST CONCRETE MEMBERS	INSPECT ALL CONNECTIONS OF THE PRECAST ELEMENTS TO THE FOUNDATIONS FOR CONFORMANCE TO THE CONTRACT DOCUMENTS	N	PERIODIC	ACI CHAPTER 16
ERECTION OF PRECAST CONCRETE MEMBERS	INSPECT ALL CONNECTIONS OF THE PRECAST ELEMENTS TO THE STRUCTURAL FRAMING FOR CONFORMANCE TO THE CONTRACT DOCUMENTS	N	PERIODIC	ACI CHAPTER 16
IN-SITU CONCRETE STRENGTH - PRE-STRESSING	VERIFY IN-PLACE CONCRETE STRENGTH PRIOR TO STRESSING TENDONS IN POST-TENSIONED CONCRETE	N	PERIODIC	ACI 318 SECTION 6.2
IN-SITU CONCRETE STRENGTH - FORMS AND SHORES	VERIFY IN-PLACE CONCRETE STRENGTH PRIOR TO REMOVAL OF FORMS AND SHORES FROM BEAMS AND STRUCTURAL SLABS	N	PERIODIC	ACI 318 SECTION 6.2
FORMWORK	INSPECT FORMWORK FOR SHAPE, LOCATION AND DIMENSIONS OF THE CONCRETE MEMBER BEING FORMED	Y	PERIODIC	ACI 318 SECTION 6.1.
WATER STOPS	VERIFY ALL WATER STOPS ARE PROPERLY INSTALLED AND ANCHORED INTO POSITION PRIOR TO PLACEMENT OF CONCRETE	N	PERIODIC	

SPECIAL INSPECTION SERVICES SCHEDULE - COLD-FORMED STEEL DECK

	REFER TO IBC 2012, CHAPTER 17			<u> </u>
ITEM	TASK	APPLICABLE TO PROJECT (Y/N)	FREQUENCY	REFERENCE
PRIOR TO DECK PLACEMENT:				
MATERIAL COMPLIANCE	VERIFY COMPLIANCE OF DECK AND ALL DECK ACCESSORIES WITH CONSTRUCTION DOCUMENTS, INCLUDING PROFILES, MATERIAL PROPERTIES AND BASE METAL THICKNESS	Y	PERIODIC	AWS D1.3, SDI C, NC AND R (AS APPLICABLE)
MATERIAL ACCEPTANCE	DOCUMENT ACCEPTANCE OR REJECTION OF DECK AND DECK ACCESSORIES	Y	PERIODIC	
AFTER DECK PLACEMENT:				
STEEL DECK INSTALLATION	VERIFY PROPER INSTALLATION OF STEEL DECK AND ACCESSORIES	Y	PERIODIC	SDI C, NC AND R (AS APPLICABLE)
MATERIAL MILL CERTIFICATIONS	VERIFY DECK MATERIALS ARE REPRESENTED BY APPROPRIATE MILL CERTIFICATIONS	Y	PERIODIC	
INSTALLATION ACCEPTANCE	DOCUMENT ACCEPTANCE OR REJECTION OF DECK AND DECK ACCESSORIES INSTALLATION	Y	PERIODIC	
PRIOR TO WELDING:				
WELDING PROCEDURE SPECIFICATION(S)	VERIFY WELDING PROCEDURE SPECIFICATION (WPS) AVAILABLE	Y	PERIODIC	AWS D1.3, SDI C, NC AND R (AS APPLICABLE)
WELDING CONSUMABLES MANUFACTURER CERTIFICATIONS	VERIFY MANUFACTURER'S CERTIFICATIONS ARE AVAILABLE FOR WELDING CONSUMABLES	Y	PERIODIC	AWS D1.3, SDI C, NC AND R (AS APPLICABLE)
WELDING MATERIAL TYPE/GRADE	VERIFY WELDING MATERIAL TYPE/GRADE	Y	PERIODIC	AWS D1.3, SDI C, NC AND R (AS APPLICABLE)
WELDING EQUIPMENT	VERIFY WELDING EQUIPMENT IN GOOD WORKING ORDER	Y	PERIODIC	AWS D1.3, SDI C, NC AND R (AS APPLICABLE)
DURING WELDING:				
QUALIFIED WELDERS	VERIFY USE OF QUALIFIED WELDERS	Y	PERIODIC	AWS D1.3
WELDING CONSUMABLES	VERIFY PROPER CONTROL AND HANDLING OF WELDING CONSUMABLES	Y	PERIODIC	AWS D1.3
ENVIRONMENTAL CONDITIONS	DOCUMENT ENVIRONMENTAL CONDITIONS (WIND SPEED, MOISTURE, TEMPERATURE) ARE ACCEPTABLE	Y	PERIODIC	AWS D1.3
WPS FOLLOWED	VERIFY PROPER WPS IS FOLLOWED DURING WELDING	Y	PERIODIC	AWS D1.3
AFTER WELDING:				
FIELD WELDING	INSPECT FIELD WELDS FOR PROPER SIZE, LOCATION, PATTERN, AND SPACING FOR ALL WELDS	Y	PERIODIC	AWS D1.3, SDI C, NC AND R (AS APPLICABLE)
VISUAL ACCEPTANCE	VERIFY WELDS MEET VISUAL ACCEPTANCE CRITERIA	Y	PERIODIC	AWS D1.3, SDI C, NC AND R (AS APPLICABLE)
REPAIR ACTIVITIES	VERIFY DEFICIENT WELDS ARE PROPERLY REPAIRED	Y	PERIODIC	AWS D1.3, SDI C, NC AND R (AS APPLICABLE)
WELD ACCEPTANCE	DOCUMENT ACCEPTANCE OR REJECTION OF WELDS	Y	PERIODIC	AWS D1.3, SDI C, NC AND R (AS APPLICABLE)
PRIOR TO MECHANICAL FASTENING:				
MANUFACTURER INSTRUCTIONS	VERIFY MANUFACTURER INSTALLATION INSTRUCTIONS ARE AVAILABLE FOR MECHANICAL FASTENERS	Y	PERIODIC	SDI C, NC AND R (AS APPLICABLE)
PROPER TOOLS	VERIFY PROPER TOOLS AVAILABLE FOR FASTENER INSTALLATION	Y	PERIODIC	SDI C, NC AND R (AS APPLICABLE)
FASTENER STORAGE	VERIFY PROPER STORAGE FOR MECHANICAL FASTENERS	Y	PERIODIC	SDI C, NC AND R (AS APPLICABLE)
DURING MECHANICAL FASTENING:				
POSITIONING	VERIFY FASTENERS ARE POSITIONED AS REQUIRED (INCLUDING PROPER PATTERN, SPACING, ETC.)	Y	PERIODIC	SDI C, NC AND R (AS APPLICABLE)
INSTALLATION AFTER MECHANICAL	VERIFY FASTENERS ARE INSTALLED IN ACCORDANCE WITH MANUFACTURER'S INSTRUCTIONS	Y	PERIODIC	SDI C, NC AND R (AS APPLICABLE)
AFTER MECHANICAL FASTENING:				
SUPPORT FASTENERS	VERIFY PROPER PATTERN, SPACING, TYPE AND INSTALLATION	Y	PERIODIC	SDI C, NC AND R (AS APPLICABLE)
PERIMETER FASTENERS	VERIFY PROPER PATTERN, SPACING, TYPE AND INSTALLATION	Y	PERIODIC	SDI C, NC AND R (AS APPLICABLE)
SIDELAP FASTENERS	VERIFY PROPER SPACING, TYPE AND INSTALLATION	Y	PERIODIC	SDI C, NC AND R (AS APPLICABLE)
REPAIR ACTIVITIES	VERIFY DEFICIENT FASTENERS ARE PROPERLY REPAIRED	Y	PERIODIC	SDI C, NC AND R (AS APPLICABLE)
FASTENER ACCEPTANCE	DOCUMENT ACCEPTANCE OR REJECTION OF FASTENERS	Y	PERIODIC	SDI C, NC AND R (AS APPLICABLE)

1. "PERIODIC" REQUIRES OBSERVATION ON A RANDOM BASIS. OPERATIONS NEED NOT BE DELAYED PENDING THESE INSPECTIONS.

"CONTINUOUS" REQUIRES TASKS TO BE CONTINUOUSLY PERFORMED FOR EACH OPERATION, CONNECTION OR MEMBER AS APPLICABLE.
 INSPECT FOR CONFORMANCE WITH CONSTRUCTION DOCUMENTS, INSTALLATION DRAWINGS, SHOP DRAWINGS, DESIGN DOCUMENTS, MANUFACTURER'S INSTRUCTIONS, AND APPLICABLE REFERENCED STANDARDS.

SPECIAL INSPECTION

THE OWNER OR OWNER'S AGENT SHALL EMPLOY INDEPENDENT AGENCY(IES) OR INDIVIDUAL(S) TO PROVIDE SPECIAL INSPECTION FOR ITEMS AS INDICATED ON THE DRAWINGS.

SPECIAL INSPECTION IS A MANDATORY REQUIREMENT FOR VERIFYING CONFORMANCE OF THE INDICATED CONSTRUCTION. SPECIAL INSPECTION IS REQUIRED IN ADDITION TO ALL MATERIAL TESTS AND INSPECTIONS IDENTIFIED ELSEWHERE IN THE CONSTRUCTION DOCUMENTS.

THE SPECIAL INSPECTOR SHALL BE A QUALIFIED PERSON, WHO SHALL DEMONSTRATE COMPETENCE, TO THE SATISFACTION OF THE BUILDING OFFICIAL AND THE STRUCTURAL ENGINEER, FOR INSPECTION OF EACH PARTICULAR TYPE OF CONSTRUCTION OR OPERATION REQUIRING SPECIAL INSPECTION.

"PERIODIC" SPECIAL INSPECTION IS DEFINED AS "THE PART-TIME OR INTERMITTENT OBSERVATION OF WORK REQUIRING SPECIAL INSPECTION BY AN APPROVED SPECIAL INSPECTOR WHO IS PRESENT IN THE AREA WHERE THE WORK HAS BEEN OR IS BEING PERFORMED AND AT THE COMPLETION OF THE WORK."

"CONTINUOUS" SPECIAL INSPECTION IS DEFINED AS "THE <u>FULL-TIME OBSERVATION</u> OF WORK REQUIRING SPECIAL INSPECTION BY AN APPROVED SPECIAL INSPECTOR WHO IS PRESENT IN THE AREA WHERE THE WORK IS BEING PERFORMED."

SUBMIT TO THE STRUCTURAL ENGINEER FOR REVIEW A MINIMUM OF 14 DAYS PRIOR TO COMMENCEMENT OF CONSTRUCTION OF ELEMENTS REQUIRING SPECIAL INSPECTION THE FOLLOWING:

- SPECIAL INSPECTION THE FOLLOWING:

 1. NAME(S), ADDRESS(ES), TELEPHONE NUMBER(S), EMAIL ADDRESS(ES), AND
- STATEMENT(S) OF QUALIFICATIONS OF ALL SPECIAL INSPECTOR(S) TO BE ENGAGED ON THE PROJECT.

 2. A LISTING OF ALL ITEMS TO RECEIVE SPECIAL INSPECTION, DESIGNATION WHETHER INSPECTIONS WILL BE CONTINUOUS OR PERIODIC AND THE NAME OF THE INDIVIDUAL THAT WILL BE PERFORMING INSPECTION FOR

THE CONTRACTOR SHALL COORDINATE WITH THE SPECIAL INSPECTOR SUFFICIENTLY IN ADVANCE OF WORK REQUIRING SPECIAL INSPECTION AND SHALL PROVIDE ACCESS TO THE SITE AND TO THE CONSTRUCTION DOCUMENTS (CURRENT DRAWINGS AND SPECIFICATIONS) FOR THE SPECIAL INSPECTOR CARRY OUT THE REQUIRED OPERATIONS.

THE SPECIAL INSPECTOR SHALL OBSERVE THE WORK REQUIRING SPECIAL INSPECTION FOR CONFORMANCE TO THE CONSTRUCTION DOCUMENTS. ALL NON-CONFORMING WORK SHALL BE BROUGHT TO THE IMMEDIATE ATTENTION OF THE CONTRACTOR FOR CORRECTION, THEN, IF UNCORRECTED, TO THE IMMEDIATE ATTENTION OF THE OWNER OR OWNER'S AGENT AND STRUCTURAL ENGINEER.

THE SPECIAL INSPECTOR SHALL SUBMIT PERIODIC PROGRESS REPORTS TO THE OWNER OR OWNER'S AGENT, CONTRACTOR AND STRUCTURAL ENGINEER IDENTIFYING ALL SPECIAL INSPECTION OPERATIONS PERFORMED. REPORTS SHALL BE SUBMITTED NO MORE THAN 7 DAYS FOLLOWING EACH SPECIAL INSPECTION OPERATION. REPORTS SHALL IDENTIFY THE ITEM(S) INSPECTED AND AN INDICATION OF WHETHER THE INSPECTED ITEMS WERE IN CONFORMANCE WITH THE CONSTRUCTION DOCUMENTS.

AT THE COMPLETION OF ALL WORK REQUIRING SPECIAL INSPECTION, THE SPECIAL INSPECTOR SHALL SUBMIT A FINAL SIGNED REPORT TO THE OWNER OR OWNER'S AGENT, CONTRACTOR AND STRUCTURAL ENGINEER STATING WHETHER THE WORK REQUIRING SPECIAL INSPECTION WAS, TO THE BEST OF THE SPECIAL INSPECTOR'S KNOWLEDGE, IN CONFORMANCE WITH THE CONSTRUCTION DOCUMENTS.

FAILURE TO PERFORM SPECIAL INSPECTION FOR THE INDICATED CONSTRUCTION OR FAILURE TO CORRECT NON-CONFORMING WORK SHALL CONSTITUTE A BASIS FOR REJECTION OF THE WORK AND REMOVAL AND REPLACEMENT BY THE GENERAL CONTRACTOR AT NO ADDITIONAL COST TO THE OWNER, INCLUDING, BUT NOT LIMITED TO:

 THE COST OF REMOVAL AND REPLACEMENT OF ALL WORK FOR WHICH SPECIAL INSPECTION WAS REQUIRED BUT NOT PERFORMED, INCLUDING THE COST OF TESTING AND SPECIAL INSPECTION FOR THE REPLACEMENT WORK

PERFORMED AND SERVICES RELATED TO THE REPLACEMENT WORK.

THE COST OF ALL RELATED WORK MADE NECESSARY BY THE REMOVAL AND REPLACEMENT OF THE UNINSPECTED WORK PER ITEM 1 ABOVE.
 THE COST FOR DESIGN PROFESSIONAL'S SERVICES RELATED TO ALL WORK FOR WHICH SPECIAL INSPECTION WAS REQUIRED BUT NOT

PROVIDE SPECIAL INSPECTION FOR THE FOLLOWING CONSTRUCTION:
SOILS AND EARTHWORK

CONCRETE CONSTRUCTION STEEL CONSTRUCTION COLD-FORMED STEEL DECK

SEE TABLES ON THE DRAWINGS FOR SPECIAL INSPECTION PROGRAM REQUIREMENTS.

COUNT



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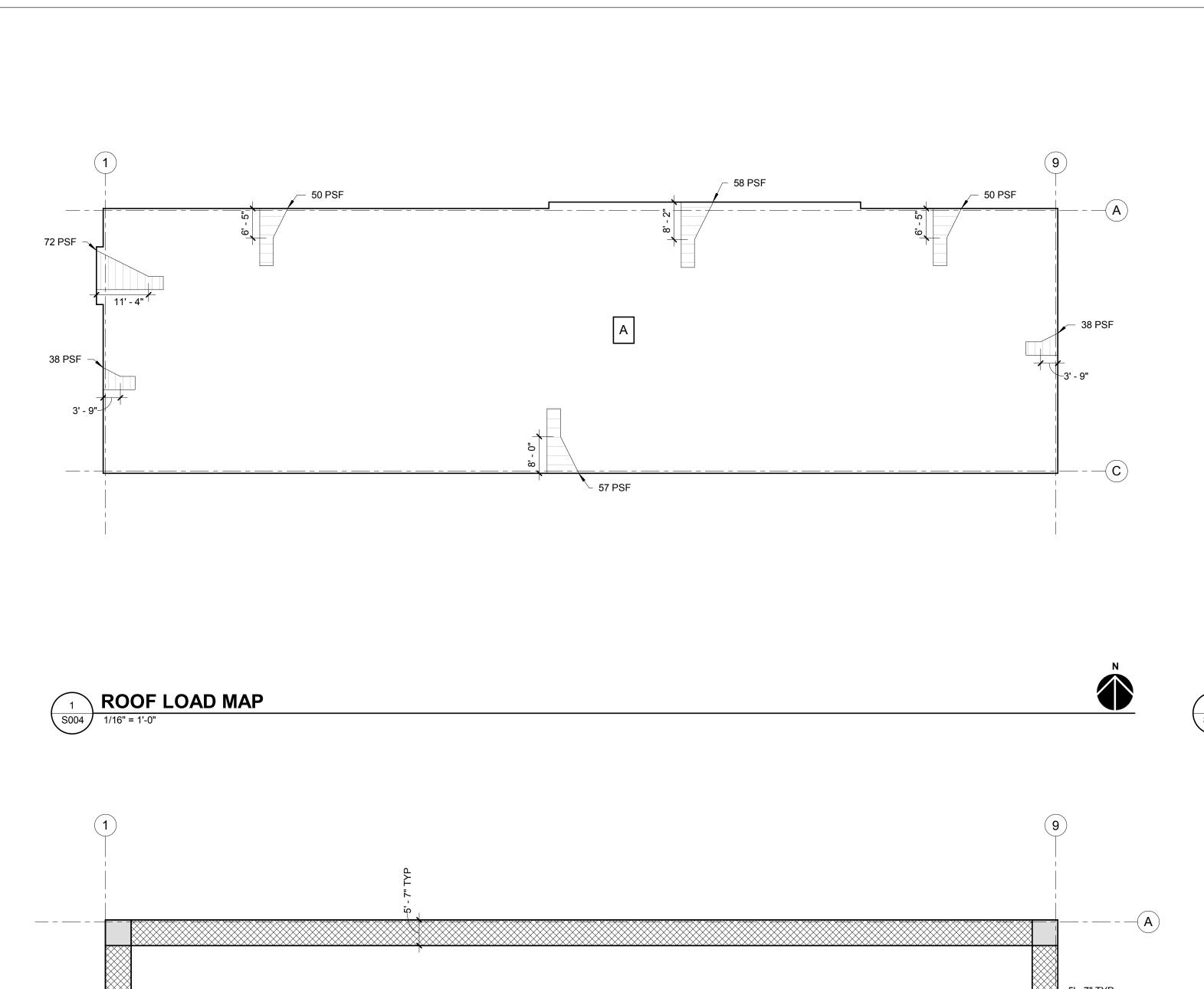
PROJECT PHASE:

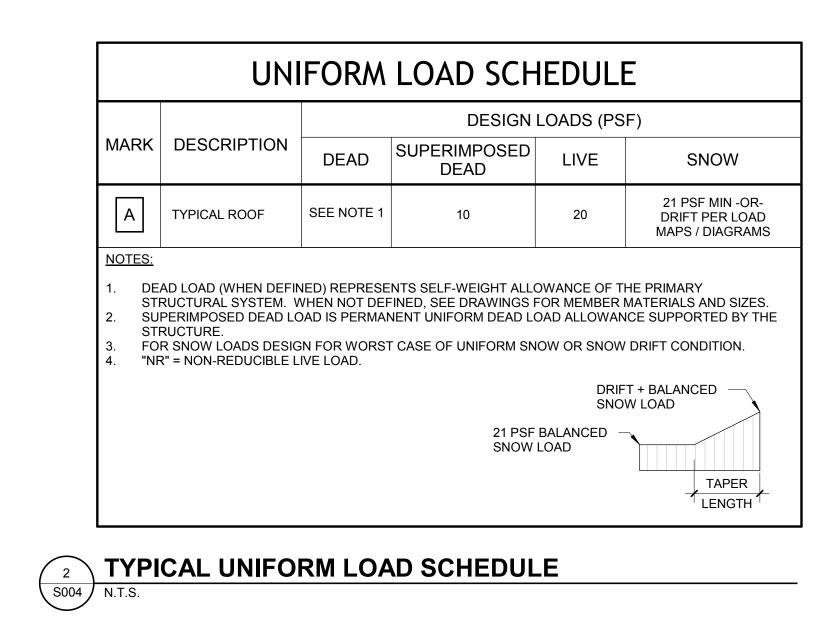
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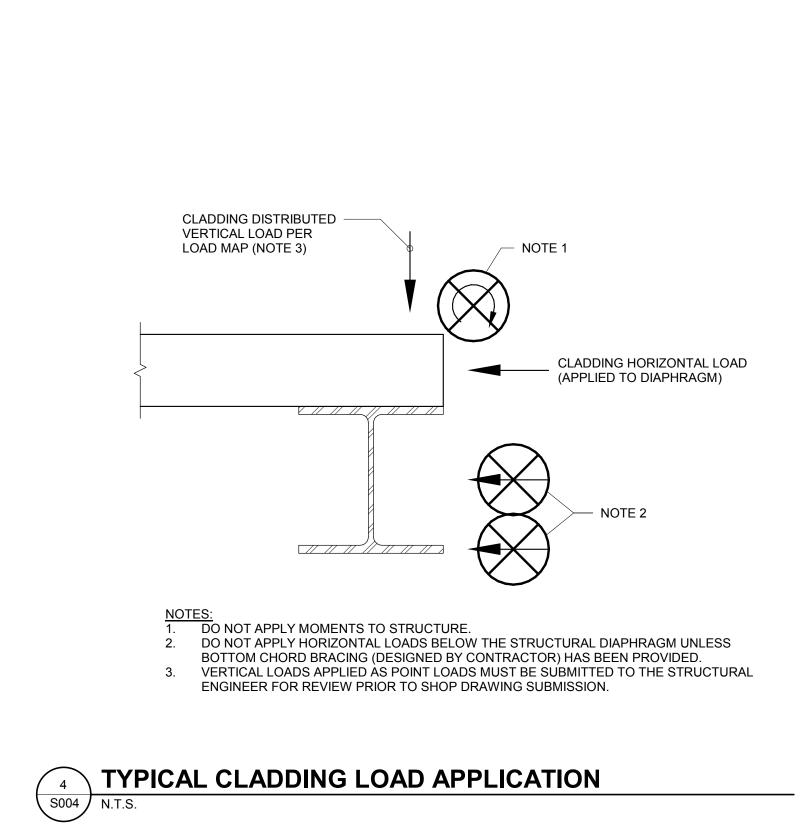
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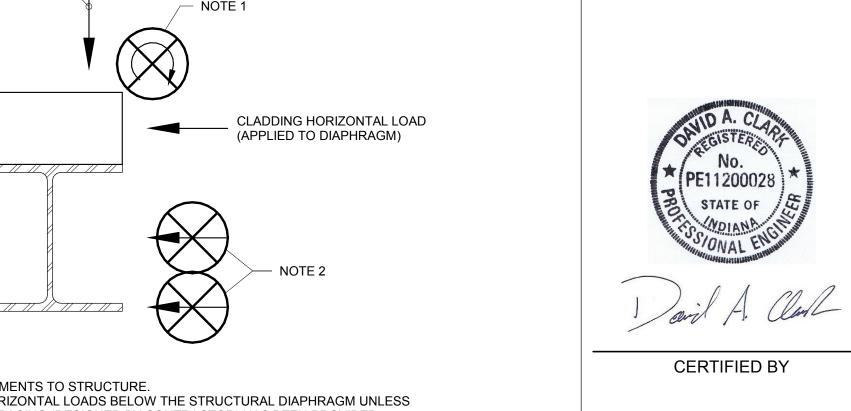
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SPECIAL INSPECTION REQUIREMENTS









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LOAD MAPS

7 SPECIAL JOIST LOAD DIAGRAM N.T.S.

- 109 PLF DL

– 109 PLF DL — 109 PLF DL ┌ 131PLF LL → 342 PLF LL ___ 109PLF DL __ 131 PLF LL 8'-0" 6'-5" SEE PLAN (DESIGN JOIST FOR MOST CRITICAL CONDITION) (DESIGN JOIST FOR MOST CRITICAL CONDITION) 20K SP1 20K SP2 5 SPECIAL JOIST LOAD DIAGRAM
N.T.S.

S004 N.T.S.

ROOF UPLIFT MAP

1 348 PLF LL - 109PLF DL $_{
m ar{}}$ 131 PLF LL 8'-2" SEE PLAN (DESIGN JOIST FOR MOST CRITICAL CONDITION) 20K SP3

6 SPECIAL JOIST LOAD DIAGRAM

1 300 PLF LL

ROOF JOIST NET UPLIFT

18.6 15.6

11.2

8.3

ZONE 1

5.4

REFERENCE ASCE 7-10, FIGURE 30.4-2B FORCES SHOWN ARE ASD WIND LOADS COMBINATION 0.6D+0.6W

EFFECTIVE WIND AREA

(SF)

≥100

ZONE 1

ZONE 3





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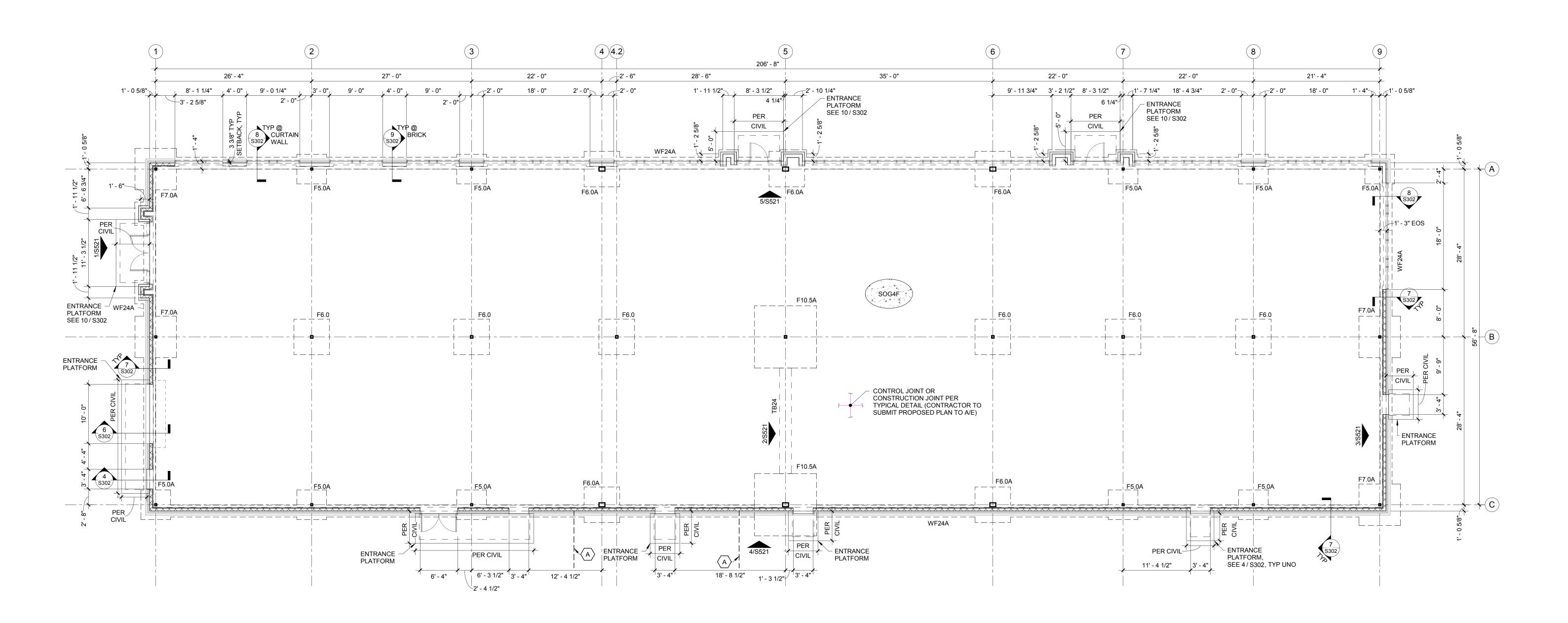


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FOUNDATION PLAN

FOUNDATION PLAN

GENERAL PLAN NOTES:

REFERENCE TOP OF SLAB (T/SLAB) = 100'-0" UNO (U.S.G.S. 642.00)
 TOP OF INTERIOR FOOTING ELEVATION = 99'-4" UNO.

3. TOP OF PERIMETER FOOTING ELEVATION = 98'-8" UNO.

4. REFER TO STRUCTURAL GENERAL NOTES, LEGEND, SCHEDULES, TYPICAL DETAILS, AND SPECIAL INSPECTION REQUIREMENTS FOR ADDITIONAL INFORMATION.

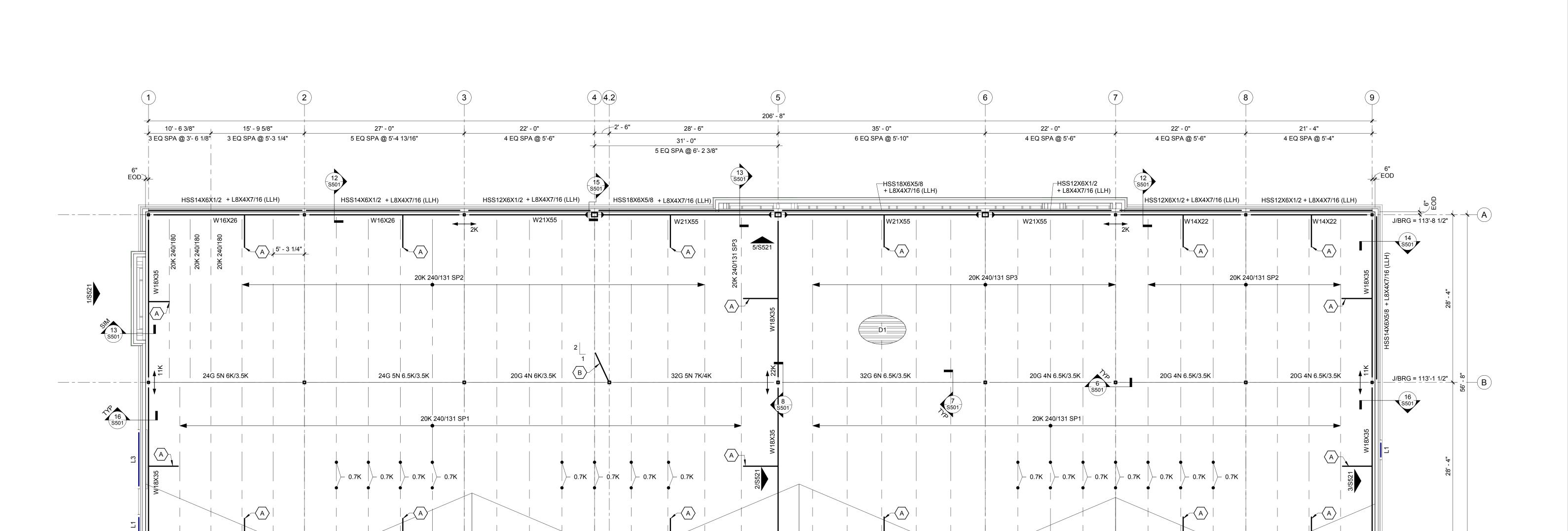
5. SEE ARCHITECTURAL DRAWINGS FOR ADDITIONAL INFORMATION AND DIMENSIONS. 6. PROVIDE ADDITIONAL SLAB ON GRADE REINFORCEMENT AT RE-ENTRANT CORNERS PER THE "TYPICAL

ADDITIONAL SLAB ON GRADE REINFORCEMENT AT RE-ENTRANT CORNERS" DETAIL.

7. PERIMETER DIMENSIONS ARE TO OUTSIDE FACE OF TURNDOWN OR CMU, UNO. 8. ALL WALL FOOTINGS ARE TYPE WF24A UNO.

A COORDINATE UTILITY ENTRANCE TO BUILDING WITH CIVIL AND MEP DRAWINGS. SEE "TYPICAL WALL FOOTING AT PIPE / CONDUIT PENETRATION DETAIL" FOR ADDITIONAL INFORMATION.

SLAB ON GRADE INFORMATION PROVIDED FOR REFERENCE ONLY. SLAB ON GRADE IS TO BE BID IN FUTURE PACKAGE.



CONTRACTOR TO INCLUDE 2 TONS OF MISCELLANEOUS STEEL IN BID FOR RTU SUPPORT AND OTHER MISCELLANEOUS FRAMING NEEDED.

L1 L1 L1

W14X22

W21X55

L1



GENERAL PLAN NOTES:

- JOIST BEARING (J/BRG) ELEVATION PER PLAN.
 REFER TO STRUCTURAL GENERAL NOTES, LEGEND, SCHEDULES, TYPICAL DETAILS, AND SPECIAL INSPECTION REQUIREMENTS FOR ADDITIONAL INFORMATION.
- SEE ARCHITECTURAL DRAWINGS FOR ADDITIONAL INFORMATION AND DIMENSIONS.
 INSTALL BRIDGING IN ACCORDANCE WITH SJI SPECIFICATIONS. BRIDGING TO RUN FULL LENGTH / WIDTH OF BUILDING.

W16X26

5. ALL JOIST GIRDER SEATS TO BE 7 1/2" DEEP, UNO.6. ALL JOIST SEATS TO BE 2 1/2" DEEP, UNO.

W16X26

PROVIDE FRAMED/REINFORCED SUPPORTS FOR ALL ROOF TOP EQUIPMENT, HATCHES, OPENINGS AND ROOF DRAINS
PER THE TYPICAL DETAILS. COORDINATE EXACT SIZES AND LOCATIONS OF ALL FRAMING WITH APPLICABLE
CONTRACTOR PRIOR TO FABRICATION.

KEYED PLAN NOTES:

- A BRACE BOTTOM FLANGE OF OF BEAM PER "BOTTOM FLANGE BRACING AT ROOF" DETAILS
- B BRACE TOP OF COLUMN WITH L3x3x1/4 ANGLE. WELD TO COLUMN FACE AND JOIST TOP CHORD WITH 1/4" WELD 3" MIN.

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J/BRG = 112'-6 3/8" C

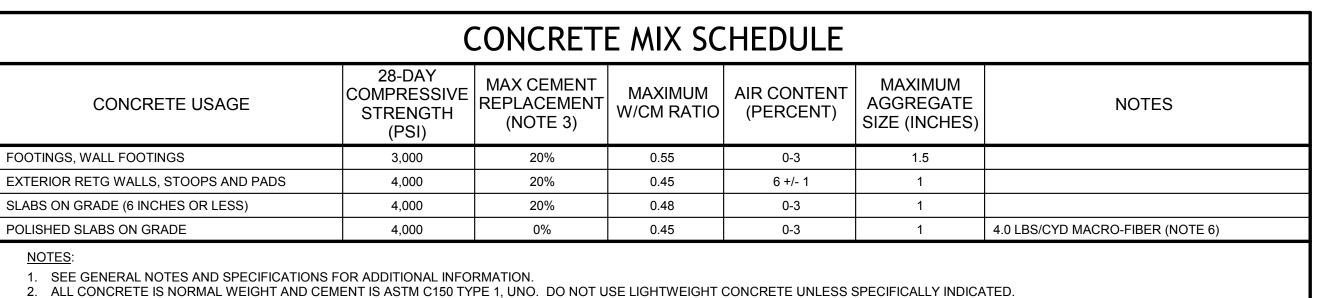
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ROOF FRAMING PLAN



3. ACCEPTABLE CEMENT REPLACEMENT MATERIAL, WHERE PERMITTED, SHALL BE FLY ASH, ASTM C618 TYPE C OR F, UNO.

f'c = 4000 PSI

Ld Ldt Lt

15 | 20 | 20 | 26

24 | 32 | 32 | 41

29 | 38 | 38 | 50

BAR SIZE

#10

- TARGET SLUMP SHALL BE DETERMINED BY THE CONTRACTOR AS NEEDED FOR PROPER PLACEMENT.
- WHERE NOTED, BLENDED AGGREGATE WITH ZONE 2 COARSENESS PER ACI 302 IS MANDATORY. 6. COORDINATE LOCATIONS OF ALL POLISHED CONCRETE SLABS (WHEN USED) AND REVIEW THE CONCRETE MIX REQUIREMENTS WITH THE POLISHED CONCRETE CONTRACTOR PRIOR TO SUBMITTAL OF CONCRETE MIXES. IF THE POLISHED CONCRETE CONTRACTOR REQUESTS TO DEVIATE FROM THE REQUIREMENTS OF THIS SCHEDULE, CONTACT THE STRUCTURAL ENGINEER TO REVIEW THE REQUESTS PRIOR TO SUBMISSION OF THE POLISHED CONCRETE MIX(ES)

CRITERIA ARE <u>NOT</u> MET:

CONCRETE MIX SCHEDULE

BAR SIZE

#6

#7

#10

#11

NOTES:

f 'c = 3000 PSI

Ld Ldt Lt Ltt

28 | 37 | 37 | 48

33 | 43 | 43 | 56

48 | 63 | 63 | 82

55 | 72 | 72 | 93

69 | 90 | 90 | 117

76 | 99 | 99 | 129

Ldt = DEVELOPMENT LENGTH OF TOP BARS IN TENSION

Ltt = TENSION LAP SPLICE LENGTH OF TOP BARS

Lc = TIED COLUMN LAP SPLICE IN COMPRESSION

Lcs = SPIRAL COLUMN LAP SPLICE IN COMPRESSION

REBAR DEVELOPMENT/SPLICE LENGTHS ARE BASED

ON ACI 318. REINFORCEMENT YIELD STRENGTH, Fy = 60 KSI.

LARGER DIAMETER SPLICE LENGTHS GOVERN AT BAR SIZE

"TOP BARS" = HORIZONTAL <u>BEAM, MAT, OR SLAB</u> REINFORCING WITH

. FOR LIGHTWEIGHT CONCRETE, MULTIPLY TABLE VALUES BY 1.33, UN

Lb = COMPRESSION DEVELOPMENT LENGTH

4. ALL SPLICES SHALL BE TENSION SPLICES, UNO.

I. db = NOMINAL BAR DIAMETER

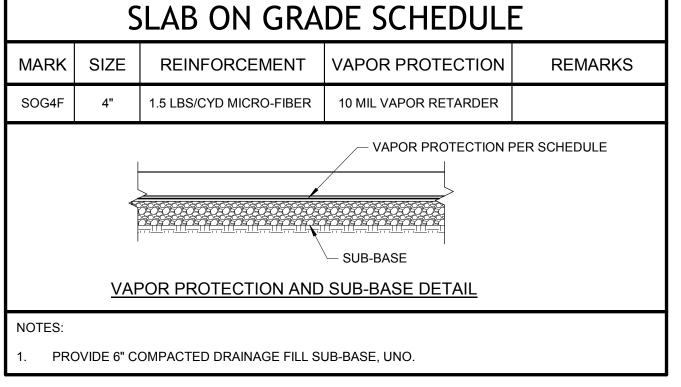
MORE THAN 12" CAST BELOW.

TRANSITIONS.

Ld = TENSION DEVELOPMENT LENGTH

Lt = TENSION LAP SPLICE LENGTH

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SLAB ON GRADE

FLATNESS / LEVELNESS SCHEDULE

15

20

25

30

OVERALL | MIN LOCAL | MIN LOCAL

20

25

40

REQUIRED SLAB

FLAT

MODERATELY FLAT

MODERATELY FLAT

FLAT

FLAT

VERY FLAT

SUPER FLAT

VERY FLAT

SUPER FLAT

SUPER FLAT

SLAB-ON-GRADE SCHEDULE

OVERALL

FF

20

25

35

45

FLOOR TYPE / LOCATION

FLATNESS/LEVELNESS SCHEDULE

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CLASSIFICATION

CONVENTIONAL

VERY FLAT

MODERATELY FLAT

COLUMN FOOTING SCHEDULE REINFORCEMENT MARK FTG SIZE (5) #5 EW BOT 5'-0" x 5'-0" x 2'-2" F6.0 6'-0" x 6'-0" x 1'-0" (6) #5 EW BOT F6.0A (9) #6 EW BOT 6'-0" x 6'-0" x 2'-2" (10) #6 EW BOT 7'-0" x 7'-0" x 2'-2" (11) #5 TOD 9 (16) #7 EW/ DC

MARK

WALL FOOTING

PER PLAN

F10.5A	10'-6" x 10'-6" x 3'-0" (11) #5 TOP & (16) #7 EW BOT	
	WALL FOOTING COUPDING	
	WALL FOOTING SCHEDULE	

FTG REINF

REMARKS

REMARKS

- SLEEVE FOR PIPE, 2" DIA LARGER

SEAL VOID WITH PERMANENT

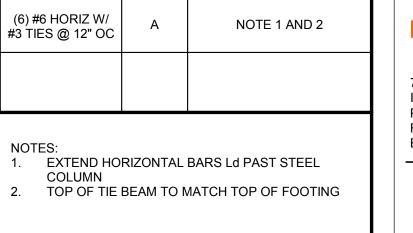
COMPRESSIBLE FILLER

LEAN CONCRETE OR

FLOWABLE FILL, SAME WIDTH

THAN PIPE, CENTERED

TIE BEAM SCHEDULE							
MARK	TIE BEAM SIZE W x D	TIE BEAM REINF	TYPE	REMARKS			
TB24	24" x 26"	(6) #6 HORIZ W/ #3 TIES @ 12" OC	Α	NOTE 1 AND 2			
TYPES:		NOTES: 1. EXTEND HO	RIZONTAL	BARS Ld PAST STEEL			



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WF24A 2'-0" x 2'-2" (2) #5 CONT BOTTOM

COLUMN AND WALL FOOTING SCHEDULES S301 N.T.S.

1' - 6"

PENETRATION BELOW FOOTING

1' - 6"

⊕ CL OF PIPE

SIZE (W x D)

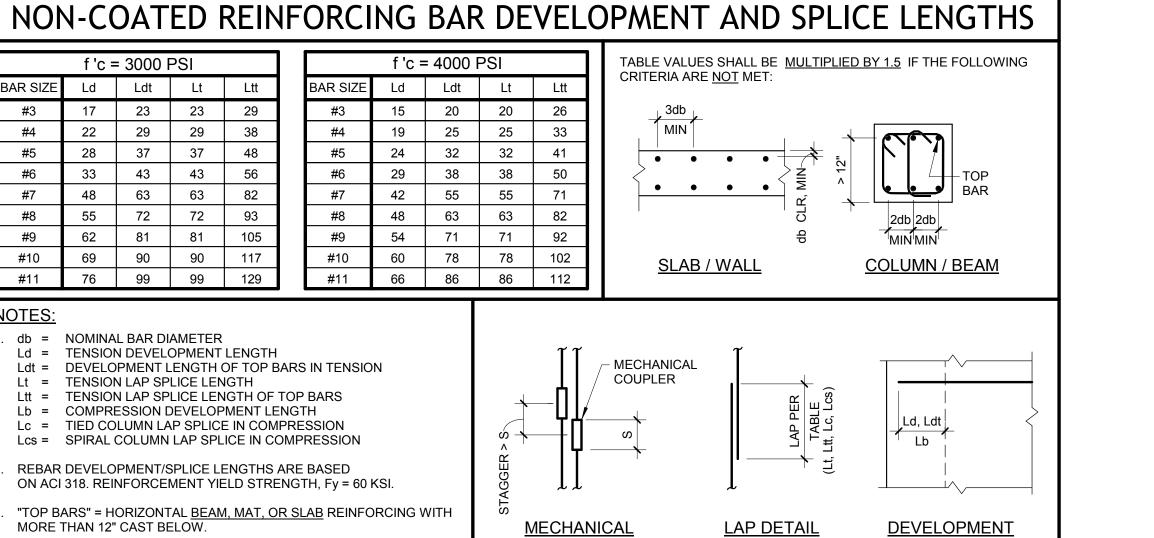
TIE BEAM SCHEDULE S301 N.T.S.

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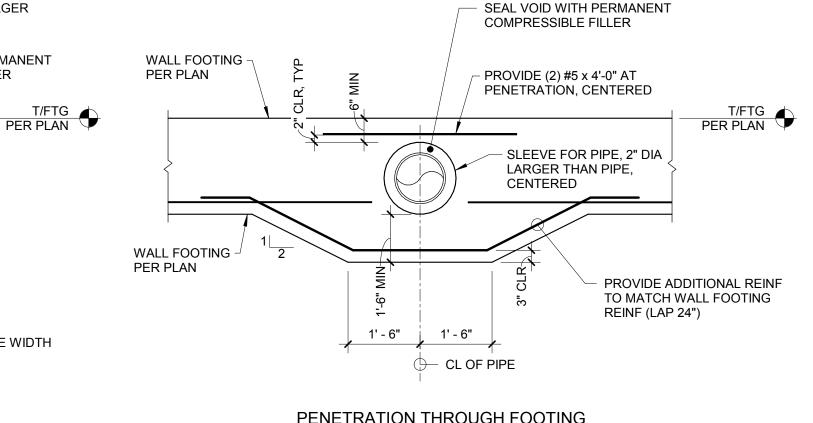
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42 | 55 | 55 | 71 SUPER FLAT 48 63 63 82 EXPOSED WAREHOUSE, MANUFACTURING AREAS, UNO 60 | 78 | 78 | 102 COLUMN / BEAM SLAB / WALL #11 66 86 86 112 EXPOSED UTILITY/MECHANICAL AREAS, UNO FLOORS WITH CARPET, VCT FINISH, UNO FLOORS WITH POLISHED CONCRETE FINISH TILE UP TO 16" LONG DIMENSION, >=1/4" GROUT JOINTS - MECHANICAL TILE UP TO 16" LONG DIMENSION, 3/16" GROUT JOINTS COUPLER TILE UP TO 16" LONG DIMENSION, 1/8" GROUT JOINTS TILE >16" TO <36" LONG DIMENSION, >=1/4" GROUT JOINTS TILE >16" TO <36" LONG DIMENSION, <1/4" GROUT JOINTS TILE >36" LONG DIMENSION TYPICAL REINFORCING BAR DEVELOPMENT / SPLICE DETAILS

GENERAL CONTRACTOR SHALL REVIEW ALL FLOOR FINISH REQUIREMENTS FOR THE PROJECT AND PROVIDE CONCRETE SLAB SURFACE FINISHES IN ACCORDANCE WITH THE REQUIREMENTS OF THE SPECIFIED FLOOR FINISH MATERIALS. WHERE TOLERANCES FOR THE FLOOR FINISH MATERIALS DIFFER FROM THIS SCHEDULE. THE MORE STRINGENT REQUIREMENTS SHALL APPLY. GENERAL CONTRACTOR SHALL COORDINATE WITH THE FINISH FLOORING SUPPLIER TO PROVIDE ALL NECESSARY REPAIR, GRINDING, AND / OR LEVELING OF THE CONCRETE SLAB TO ACCOMMODATE ALL FLOOR FINISHES PRIOR TO INSTALLATION OF FINISH MATERIALS WITH NO ADDITIONAL COST TO THE PROJECT.



1. SEE PLUMBING DRAWINGS FOR LOCATION AND SIZE OF PENETRATIONS. 2. DETAIL SHOWN IS FOR MAXIMUM OF 12" DIA PIPE. FOR LARGER SIZE, CONTACT STRUCTURAL ENGINEER PRIOR TO CONSTRUCTION

CONCRETE REINFORCEMENT DEVELOPMENT AND LAP SPLICE TABLE

VERTICAL REBAR SIZE

TO MATCH WALL REINF

CORNER BARS MATCH

CMU BOND BEAM UNIT

BOND BEAM REINF

W/ OPEN BOTTOM

VERTICAL REBAR SIZE

TO MATCH WALL REINF

PREFABRICATED L-SHAPE

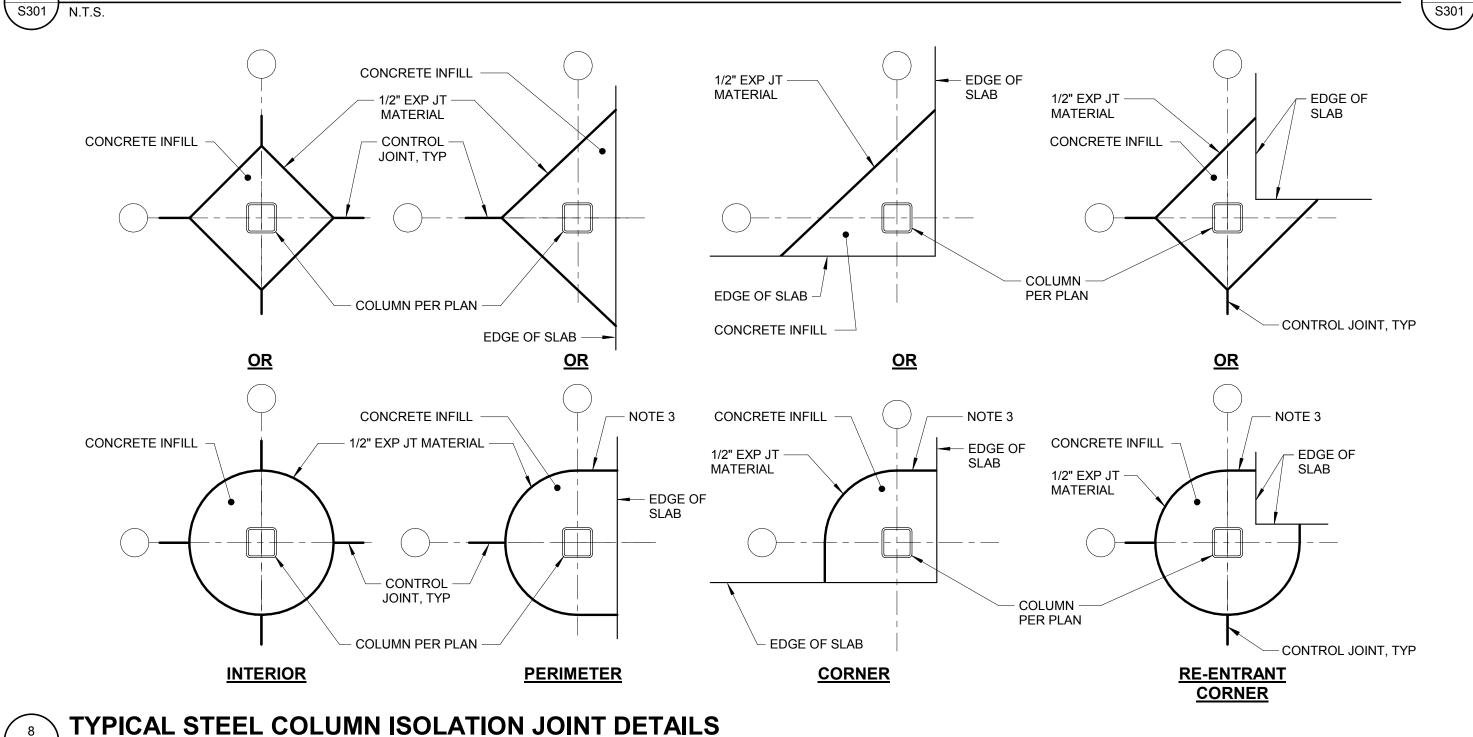
MATCH CONTINUOUS JOINT

<u>CORNER</u>

JOINT REINF AT WALL CORNER.

REINF AND LAP SPLICE 6" MIN.

BOND BEAM REINF (LAP



VERTICAL REBAR SIZE TO

MATCH WALL REINF, TYP

INTERSECTION BARS

MATCH BOND BEAM

REINF (LAP SPLICE)

BOND BEAM REINF

W/ OPEN BOTTOM

<u>INTERSECTION</u>

<u>INTERSECTION</u>

CMU BOND BEAM UNIT

BOND BEAM DETAILS

VERTICAL REBAR SIZE TO

MATCH WALL REINF, TYP

PREFABRICATED T-SHAPE

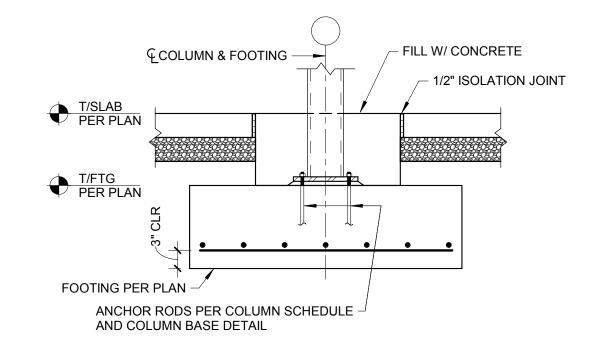
JOINT REINF AT WALL

CONTINUOUS JOINT

INTERSECTION. MATCH

REINF AND LAP SPLICE 6"

JOINT REINFORCEMENT DETAILS



SLAB ON GRADE

- 1. FOR COLUMNS PLACED AFTER SLAB-ON-GRADE, SIZE ISOLATION JOINT AS REQUIRED TO ALLOW FOR BASE PLATE INSTALLATION AND TIGHTENING OF ANCHOR RODS AND MAINTAIN NOT LESS THAN 4" MINIMUM CLEAR BETWEEN COLUMN AND ISOLATION JOINT.
- 2. FOR COLUMNS PLACED BEFORE SLAB-ON-GRADE, SIZE ISOLATION JOINT AS REQUIRED TO KEEP ALL ANCHOR RODS WITHIN THE ISOLATION JOINT AND MAINTAIN NOT LESS THAN 4" MINIMUM CLEAR BETWEEN COLUMN AND ISOLATION JOINT.
- 3. FOR CIRCULAR ISOLATION JOINTS AT PERIMETER AND CORNERS, ANGLE OF INTERSECTION BETWEEN JOINT AND WALL TO BE A MINIMUM OF 90 DEGREES TO PREVENT RETURN OF SLAB BEHIND CIRCULAR ISOLATION JOINT.

LAP SPLICE CHART

BAR SPLICE LENGTHS

48"

72"

84"

96"

109"

UNCOATED BARS EPOXY-COATED BARS

29"

58"

68"

77"

87"

54"

72"

90"

108"

126"

144"

164"

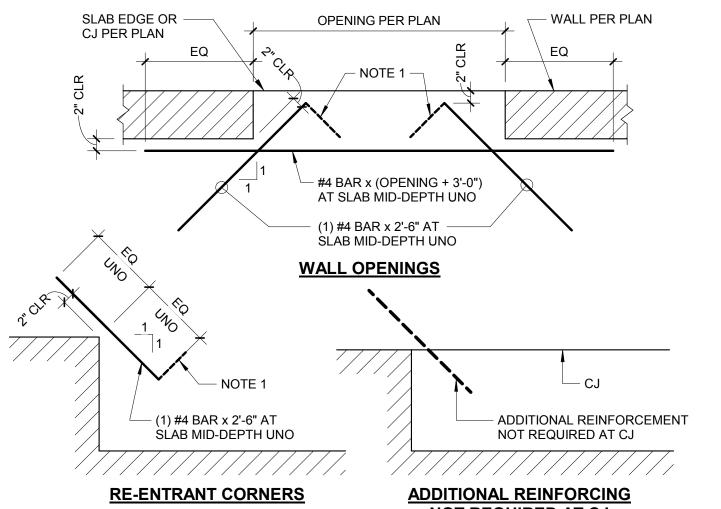
FOOTING REINF

PER SCHEDULE

S301 / N.T.S.

TYPE 1.0LD TYPE 1.5LD TYPE 1.0LD TYPE 1.5LD

TYPICAL WALL FOOTING AT PIPE / CONDUIT PENETRATION DETAIL



NOT REQUIRED AT CJ WHERE THERE IS INSUFFICIENT ROOM TO INSTALL A STRAIGHT BARAS SHOWN CENTERED WITH

PROVIDE ADDITIONAL BAR x 8'-0" FOR

CENTER ON CENTERLINE OF FOOTING

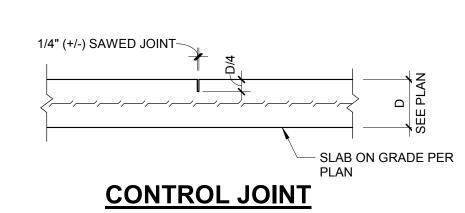
AS TYPICAL FOOTING REINFORCING.

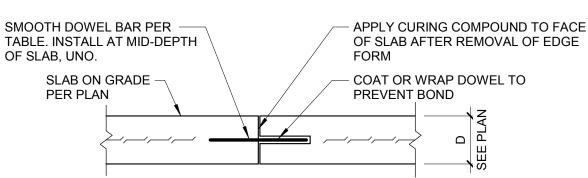
EACH BAR INTERRUPTED BY BLOCKOUT

BLOCKOUT. INSTALL AT SAME ELEVATION

EQUAL LENGTH EACH SIDE OF THE RE-ENTRANT CORNER, PROVIDE BAR WITH A 90- OR 180-DEGREE HOOK INSTALLED ON THE SIDE WITH SHORTER EMBEDMENT. FOR SLABS WITH 2 LAYERS OF REINFORCEMENT, INSTALL ADDITIONAL REINFORCEMENT AT ELEVATION OF TOP LAYER ONLY.

TYPICAL ADDITIONAL SLAB ON GRADE REINF AT RE-ENTRANT CORNERS DETAIL





CONSTRUCTION JOINT

DO\	DOWEL SIZE AND SPACING						
SLAB DEPTH (IN)	DOWEL BAR DIAMETER (IN)	TOTAL BAR LENGTH (IN)	BAR SPACING (CTR - CTR) (IN)				
4	3/4	16	24				

TYPICAL SLAB ON GRADE JOINTS DETAIL S301 N.T.S.

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PRECAST MFR NOTE:
LOCATE WALL-TO-FOOTING CONNECTIONS FORMED BLOCKOUT IN A MINIMUM OF 1'-0" AWAY FROM EDGES OF CONCRETE FOOTING BLOCKOUT. WALL FOOTING -PER PLAN FOOTING REINF - ROOF DRAIN OR PLUMBING LINE PER PER SCHEDULE ARCH, CIVIL, AND MEP DRAWINGS

WALL PER PLAN -CONCRETE CONTRACTOR NOTE: COORDINATE BLOCKOUT DEPTH AND SLEEVE REQUIREMENT THROUGH / BELOW FOOTING WITH PLUMBING AND SITE UTILITY CONTRACTORS PRIOR TO CONSTRUCTION. DO NOT INSTALL BLOCKOUTS CLOSER THAN 12'-0" OC WITHOUT PRIOR REVIEW AND APPROVAL OF THE STRUCTURAL ENGINEER.

TYPICAL WALL FOOTING BLOCKOUT AT 13 PLUMBING LINE

BOND BEAM REINF TERMINATE 2" BACK MASONRY REINFORCING STEEL FROM END OF WALL VERTICAL REBAR SIZE TO MATCH WALL REINF, TYP TERMINATE 2" BACK #9 FROM END OF WALL

VERTICAL REBAR SIZE TO

MATCH WALL REINF, TYP

JOINT REINF.

<u>END</u>

- NOTES: ALL SPLICES ARE TYPE 1.0LD, UNO. BARS LARGER THAN #9 ARE REQUIRED TO BE SPLICED BY MECHANICAL CONNECTORS, UNO.
- SPLICES BASED ON Fs = 32,000 PSI AND f'm >= 1500 PSI. ALL BARS ARE UNCOATED, UNO. USE EPOXY-COATED BARS ONLY IN PARKING STRUCTURE MASONRY WHERE SPECIFICALLY INDICATED, UNO.

MASONRY LAP SPLICE TABLE

26"

32"

45"

58"

REINFORCEMENT. TYPICAL CMU WALL JOINT DETAILS

NOTES:

S301 N.T.S.

CUT CMU WEBS AND FACE

ARE UNINTERRUPTED, TYP

SO GROUT AND REINF

1. VERTICAL REINFORCEMENT INDICATED

ADDITION TO SCHEDULED VERTICAL

REINFORCEMENT, UNLESS SCHEDULED

REINFORCEMENT ALREADY OCCURS AT

ON THIS DETAIL IS REQUIRED IN

THE INDICATED LOCATIONS.

PROVIDE LAPPED DOWELS INTO

FOUNDATION AT ALL VERTICAL

- PRE-FORMED CONTROL

JOINT GASKET IN SASH

BOND BEAM REINF TERMINATE 2" BACK FROM CONTROL JOINT EACH SIDE.

(EXCEPTION: AT BOND BEAM AT TOP OF

PERIMETER AND LOAD-BEARING WALLS

STOP CJ AT BOTTOM OF BOND BEAM

AND MAINTAIN TOP BOND BEAM AND

BLOCK CMU

- BACKER ROD AND

SEALANT EACH

REINF CONTINUOUS, UNO)

- PRE-FORMED

BLOCK CMU

CONTROL JOINT

GASKET IN SASH

- BACKER ROD AND

SEALANT EACH

SIDE

TERMINATE 2" BACK

FROM CONTROL JOINT

JOINT REINF.

EACH SIDE

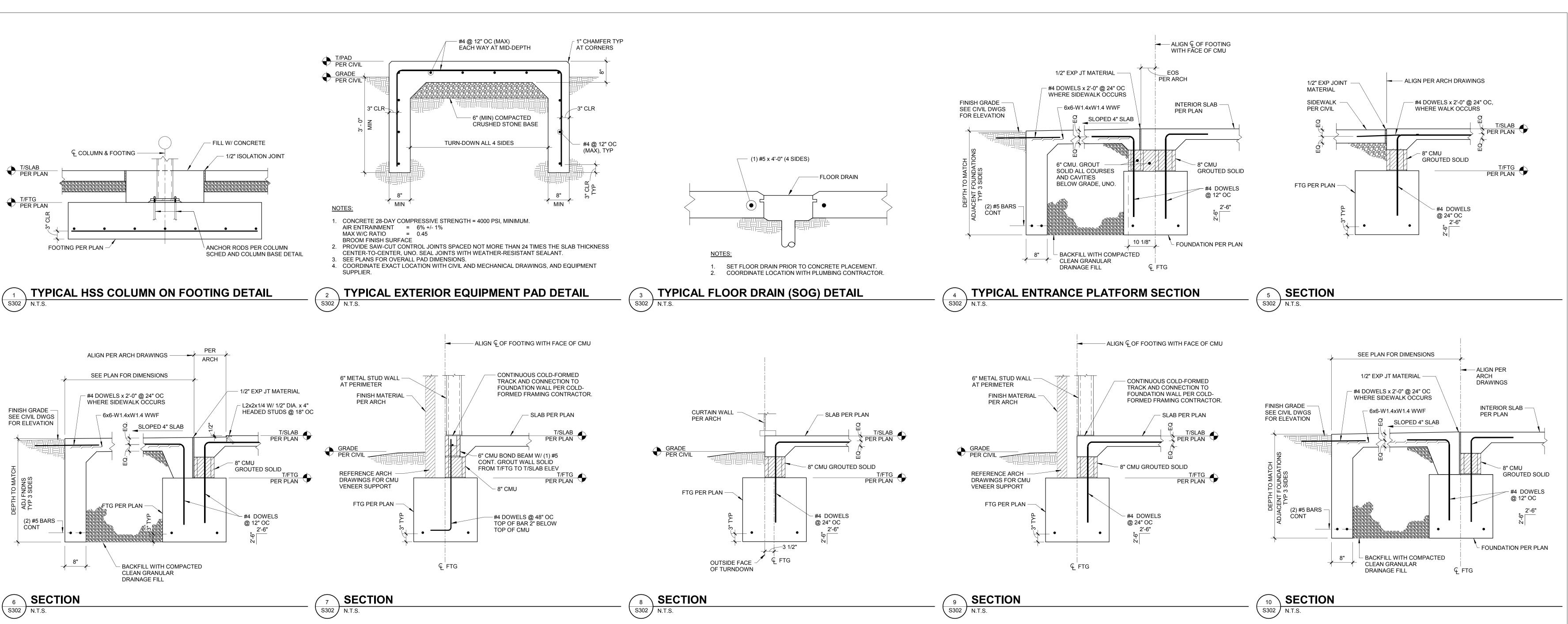
CONTROL JOINT

SIDE

CONTROL JOINT

Project Number 2017.01279 FOUNDATION

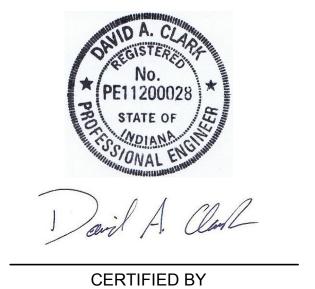
> SCHEDULES, SECTIONS, AND **DETAILS**





PORTER COUNTY - TRUSTEES OFFICE

PORTAGE, IN



ISSUANCE INDEX

DATE:

08.20.18

PROJECT PHASE:

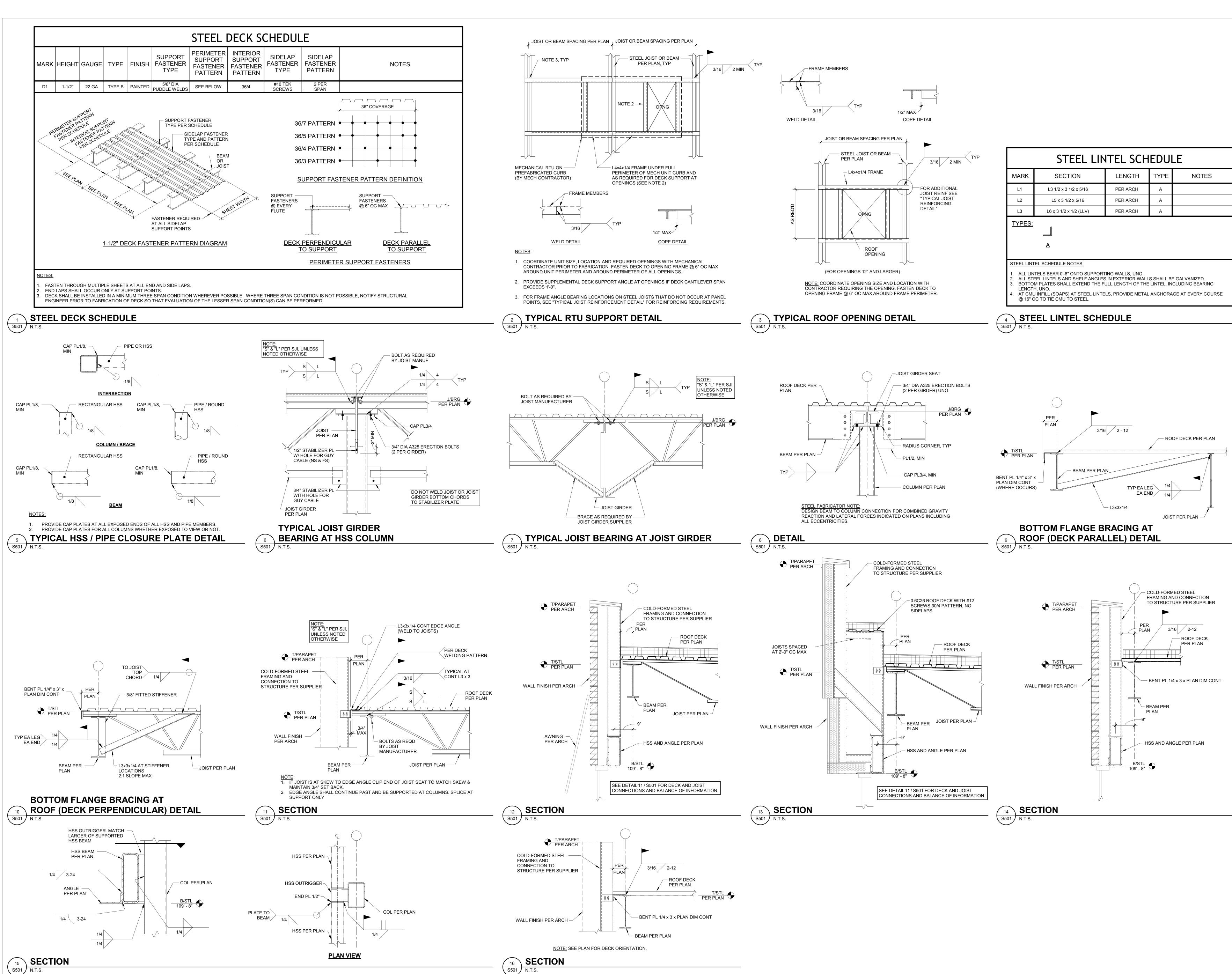
REVISION SCHEDULE

NO. DESCRIPTION DATE

100% CONSTRUCTION DOCUMENTS - BP1

Project Number 2017.01279

FOUNDATION SECTIONS AND DETAILS



STRUCTUREPOINT

7260 Shadeland Station Indianapolis, IN 46256 P: 317.547.5580 F: 317.543.0270 E: dmccloskey@structurepoint.com

SKILLMAN

8006 Aetna Street Merrillville, IN 46410 P: 219.942.2787 E: dmanderson@skillman.com

PORTER COUNTY -TRUSTEES OFFICE

PORTAGE, IN

CERTIFIED BY

ISSUANCE INDEX DATE: 08.20.18

PROJECT PHASE:

REVISION SCHEDULE

100% CONSTRUCTION DOCUMENTS - BP

NO. DESCRIPTION DATE

Project Number 2017.01279

STEEL SCHEDULES SECTIONS, AND **DETAILS**

S501

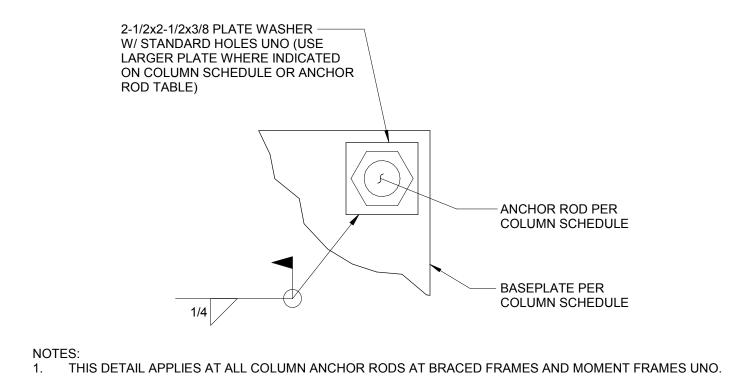
ROOF																												ROOF
						1																						
114' - 0"	HSS4X4X5/16	HSS4X4X5/16	HSS4X4X5/16	HSS12X8X5/16	HSS12X8X5/16	HSS12X8X5/16	HSS4X4X5/16	HSS4X4X5/16	HSS4X4X5/16	HSS4X4X5/16	HSS5X5X5/16	HSS4X4X5/16	HSS4X4X5/16	HSS4X4X5/16	HSS4X4X5/16	HSS12X8X5/16	HSS12X8X5/16	HSS12X8X5/16	HSS4X4X5/16	HSS4X4X5/16	HSS4X4X5/16	114' - 0" FIRST LEVEL						
100' - 0"																												100' - 0"
	BP1	BP1	BP1	BP3	BP3	BP3	BP1	BP1	BP1	BP1	BP2	BP2	BP2	BP2	BP2	BP2	BP3	BP1	BP1	BP1	BP1	BP3	BP3	BP3	BP1	BP1	BP1	
Column Locations	A-1	A-2	A-3	A-4	A-5	A-6	A-7	A-8	A-9	B-1	B-2	B-3	B-4.2	B-5	B-6	B-7	B-8	B-9	C-1	C-2	C-3	C-4	C-5	C-6	C-7	C-8	C-9	

COLUMN SCHEDULE

COLUMN PER PLAN BASE PLATE PER COLUMN SCHEDULE	B EQ EQ E S/16	THREADED NON-SHRINK GROUT SCHEDULE PER SCHEDULE LEMBED	SECTION	- TOP NUTS / WASHERS - SHIMS AND LEVELING NUTS / WASHERS - ANCHOR RODS PER COLUMN SCHEDULE - HEX NUT (NO WASHER) TACK WELD NUT TO ANCHOR ROD, TYP
SUF WA	S THE CONTRACTOR'S RE PPORT OF COLUMN BASE SHERS OR STEEL SHIMS (D CURING OF NON-SHRINK	PLATES USING L (OR COMBINATIO	EVELING PLATES, LEVE	ELING NUTS /

TYPICAL HSS COLUMN BASE DETAIL

S511 N.T.S.



TYPICAL WELDED PLATE WASHER DETAIL

ANCHOR RODS ARE ASTM F1554 GR. 36 UNO. PROVIDE WELDED PLATE WASHERS PER "TYPICAL WELDED PLATE WASHER DETAIL" AT ALL MOMENT FRAMES AND BRACED FRAMES. COLUMN BASE PLATE SCHEDULE AND ANCHOR ROD TABLE

N.T.S.

3 1/2"

COLUMN BASE PLATE SCHEDULE

DIA

ANCHOR ROD TABLE

MINIMUM WASHER THICKNESS

1/2"

5/8"

EMBED

LENGTH

MINIMUM NON-SHRINK
PROJ ABOVE GROUT
T/CONC BED THK

PROJ ABOVE T/CONC

10"

10"

REMARKS

MIN EDGE

DISTANCE, E

1 1/2"

2 1/2"

SEE DETAIL

SEE DETAIL

SEE DETAIL

ANCHOR RODS

PLATE SIZE

B" X N" X T"

ANCHOR BASEPLATE MINIMUM WASHER SIZE

1 5/16"

2 1/16"

2 5/16"

2 3/4"

BP1 10" x 10" x 3/4"

BP2 11" x 11" x 3/4"

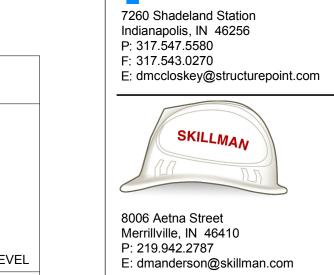
BP3 18" x 18" x 3/4"

3/4"

1 1/4"

1 1/2"

1 3/4"



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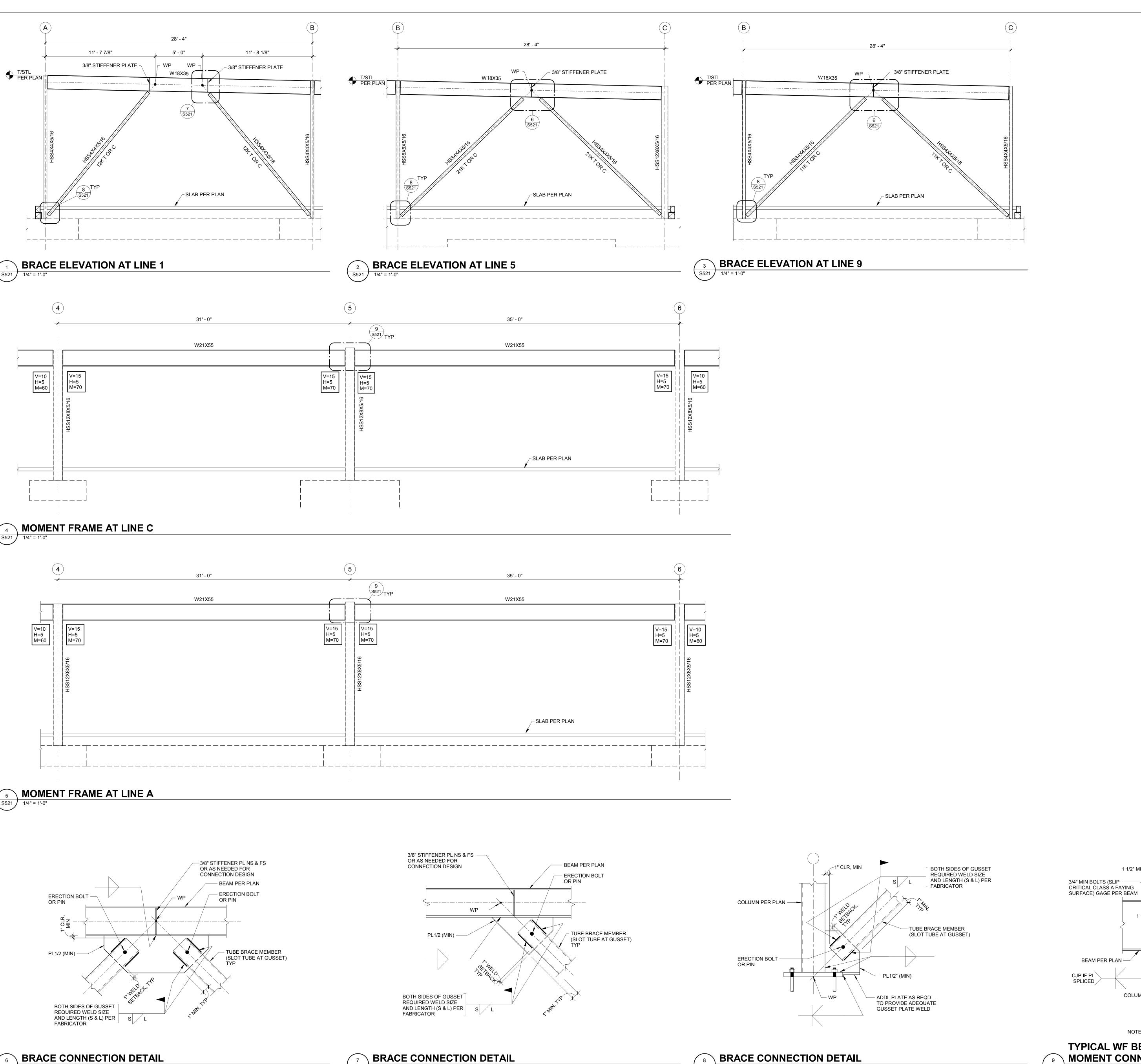
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ISSUANCE INDEX 08.20.18 PROJECT PHASE: 100% CONSTRUCTION DOCUMENTS - BP1

REVISION SCHEDULE NO. DESCRIPTION DATE

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STEEL COLUMN AND BASE PLATE SCHEDULES



BRACING GENERAL NOTES

FORCES SHOWN ARE THE MORE CRITICAL OF ALL APPLICABLE BUILDING CODE ASD LOAD

COMBINATIONS.

2. CONNECTIONS SHALL BE DESIGNED FOR BRACE FORCE INDICATED ON ELEVATION ACTING CONCURRENTLY WITH BEAM REACTION AND ANY INDICATED TRANSFER FORCES. ECCENTRICITY

SHALL BE CONSIDERED IN DESIGN OF CONNECTION.

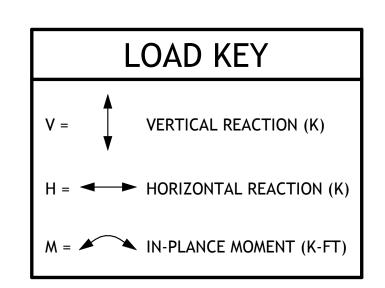
3. EACH MEMBER'S CONNECTION SHALL BE SYMMETRIC ABOUT THE CENTER OF GRAVITY OF THAT MEMBER.

4. ALL BOLTED CONNECTIONS SHALL HAVE FULLY TENSIONED HIGH STRENGTH BOLTS WITH CLASS A FAYING SURFACES.

5. SLOT IN BRACE AT GUSSET NO LARGER THAN GUSSET THICKNESS + 1/16"

6. ABBREVIATIONS: T = TENSION C = COMPRESSION

GENERAL:
ALL BRACING CONNECTIONS SHALL BE DESIGNED BY THE STEEL FABRICATOR, UNO. BRACE DETAILS INDICATED ON THE STRUCTURAL DRAWINGS ARE PROVIDED TO SHOW CONNECTION CONCEPT ONLY AND ARE NOT TO BE CONSIDERED A FINAL DESIGN. FABRICATOR'S REGISTERED PROFESSIONAL ENGINEER SHALL DESIGN AND DETAIL ALL FINAL CONNECTIONS AS REQUIRED TO SAFELY TRANSFER THE DESIGN FORCES AND ALLOW FOR FIELD FIT-UP AND ERECTION TOLERANCES.





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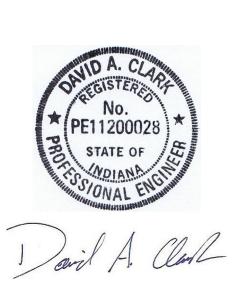
7260 Shadeland Station
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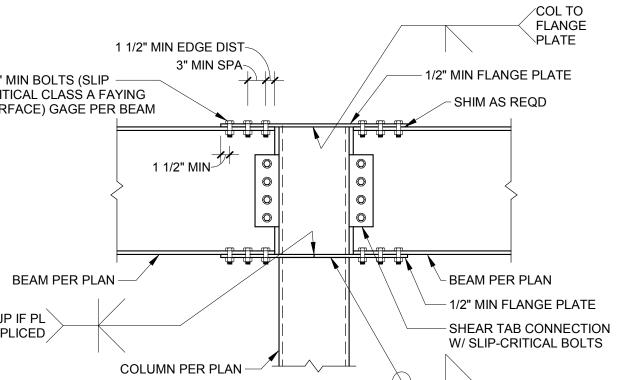
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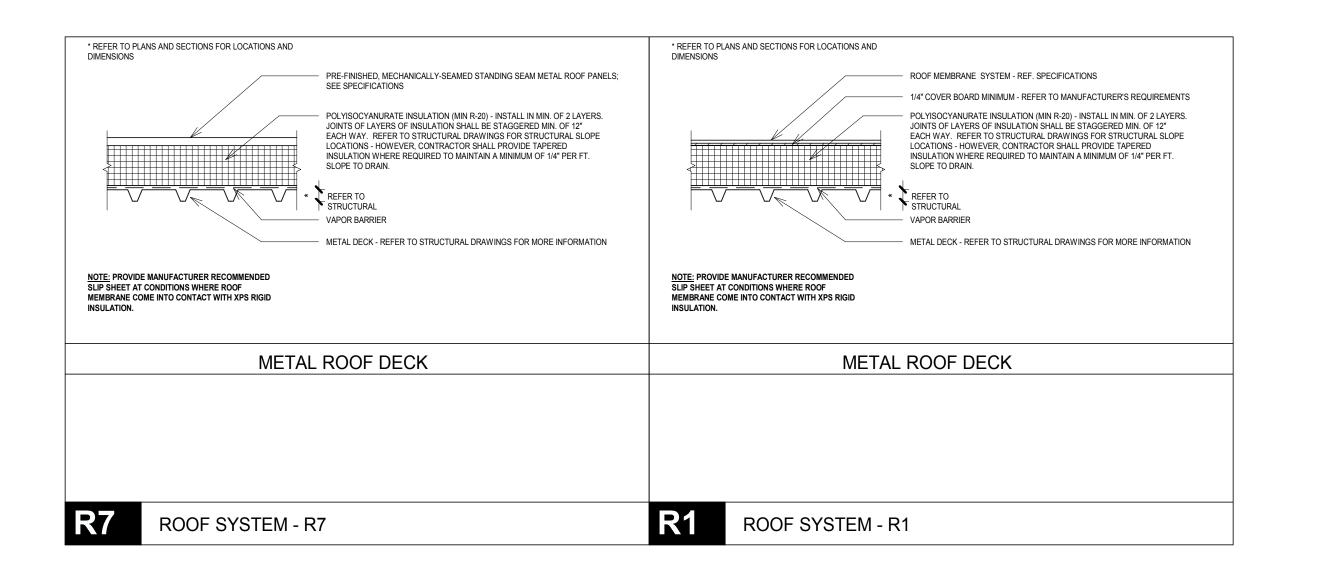
NOTE: SEE PLAN AND FRAME ELEVATIONS FOR CONNECTION FORCES.

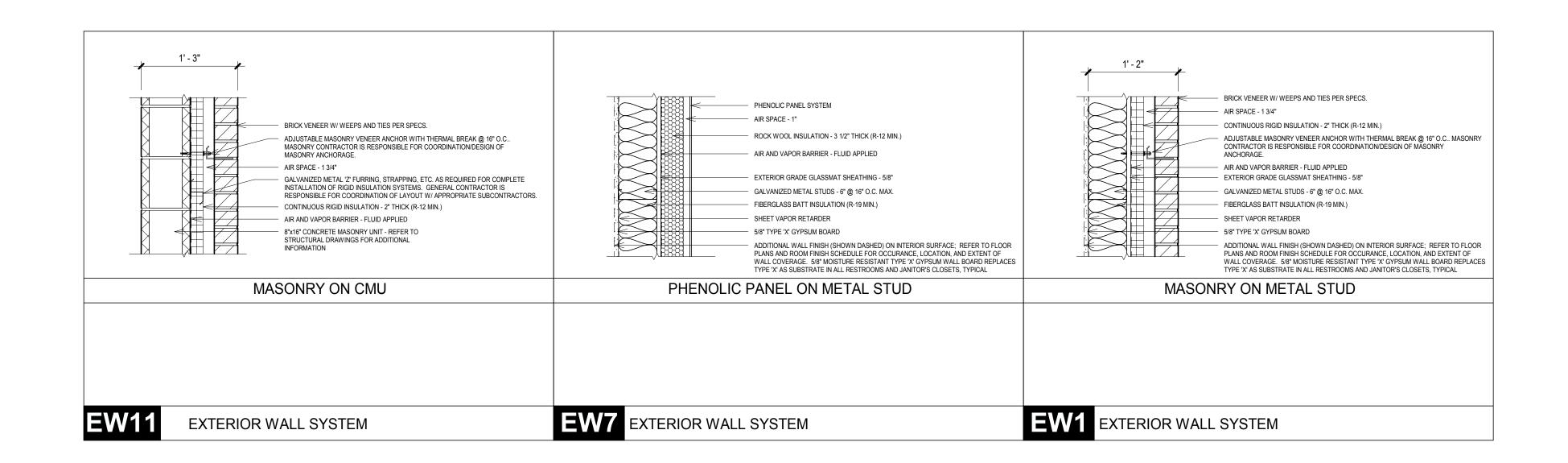
TYPICAL WF BEAM TO HSS COLUMN MOMENT CONNECTION DETAIL

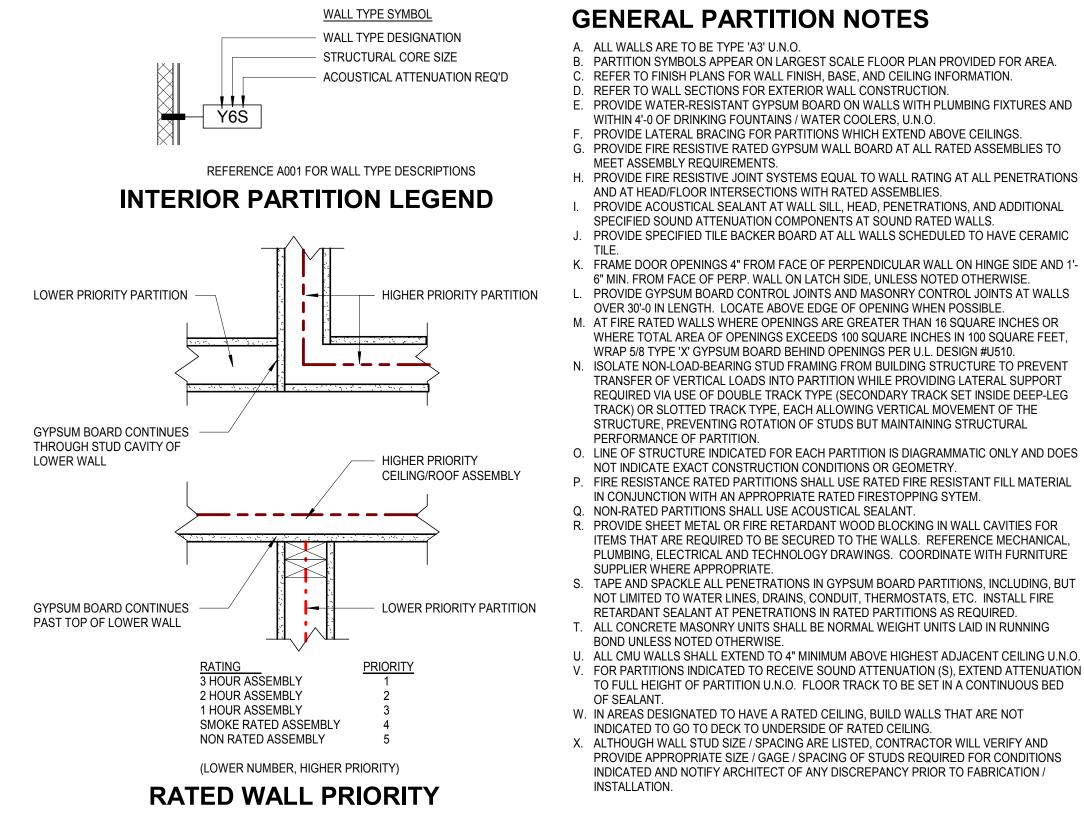
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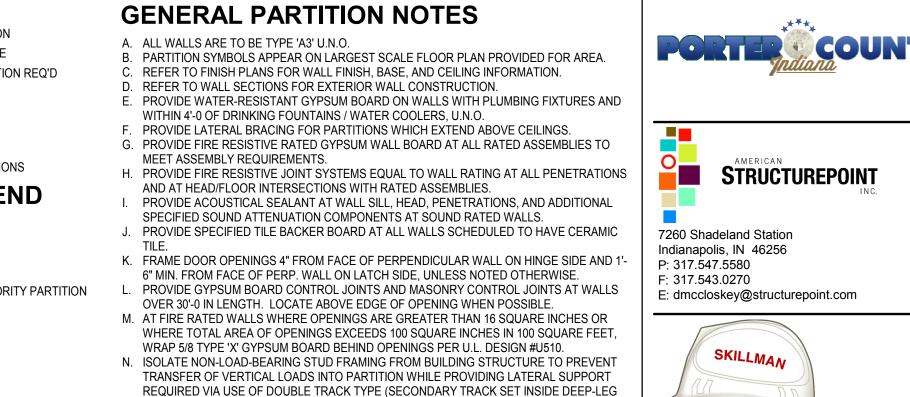
STEEL FRAME ELEVATIONS, SECTIONS AND DETAILS

S521









PERFORMANCE OF PARTITION.

SUPPLIER WHERE APPROPRIATE.

BOND UNLESS NOTED OTHERWISE.

OF SEALANT.

INSTALLATION.

NOT INDICATE EXACT CONSTRUCTION CONDITIONS OR GEOMETRY.

INDICATED TO GO TO DECK TO UNDERSIDE OF RATED CEILING.

IN CONJUNCTION WITH AN APPROPRIATE RATED FIRESTOPPING SYTEM.

NOT LIMITED TO WATER LINES, DRAINS, CONDUIT, THERMOSTATS, ETC. INSTALL FIRE

TO FULL HEIGHT OF PARTITION U.N.O. FLOOR TRACK TO BE SET IN A CONTINUOUS BED

PROVIDE APPROPRIATE SIZE / GAGE / SPACING OF STUDS REQUIRED FOR CONDITIONS

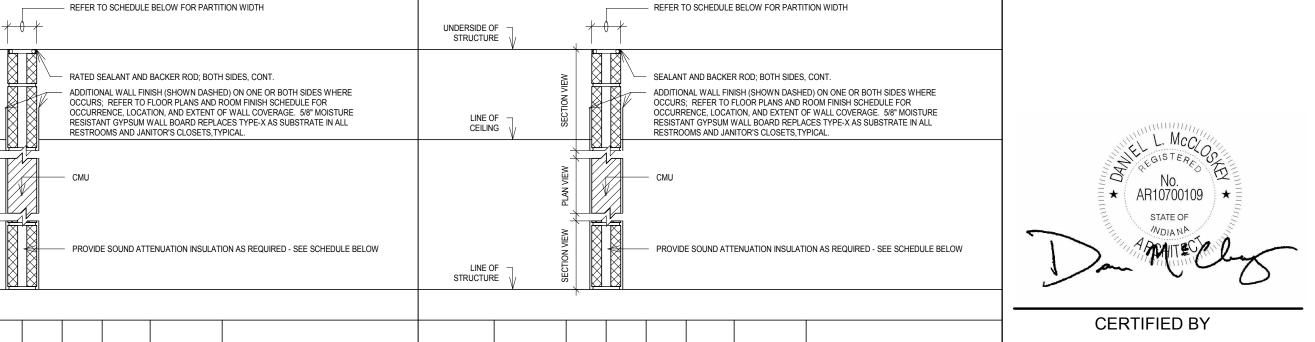
INDICATED AND NOTIFY ARCHITECT OF ANY DISCREPANCY PRIOR TO FABRICATION /

RETARDANT SEALANT AT PENETRATIONS IN RATED PARTITIONS AS REQUIRED.

TRACK) OR SLOTTED TRACK TYPE, EACH ALLOWING VERTICAL MOVEMENT OF THE STRUCTURE, PREVENTING ROTATION OF STUDS BUT MAINTAINING STRUCTURAL 8006 Aetna Street O. LINE OF STRUCTURE INDICATED FOR EACH PARTITION IS DIAGRAMMATIC ONLY AND DOES Merrillville, IN 46410 P: 219.942.2787 P. FIRE RESISTANCE RATED PARTITIONS SHALL USE RATED FIRE RESISTANT FILL MATERIAL E: dmanderson@skillman.com R. PROVIDE SHEET METAL OR FIRE RETARDANT WOOD BLOCKING IN WALL CAVITIES FOR ITEMS THAT ARE REQUIRED TO BE SECURED TO THE WALLS. REFERENCE MECHANICAL, PLUMBING, ELECTRICAL AND TECHNOLOGY DRAWINGS. COORDINATE WITH FURNITURE

3560 WILLOWCREEK RD PORTAGE, IN 46368

PORTER COUNTY ANNEX



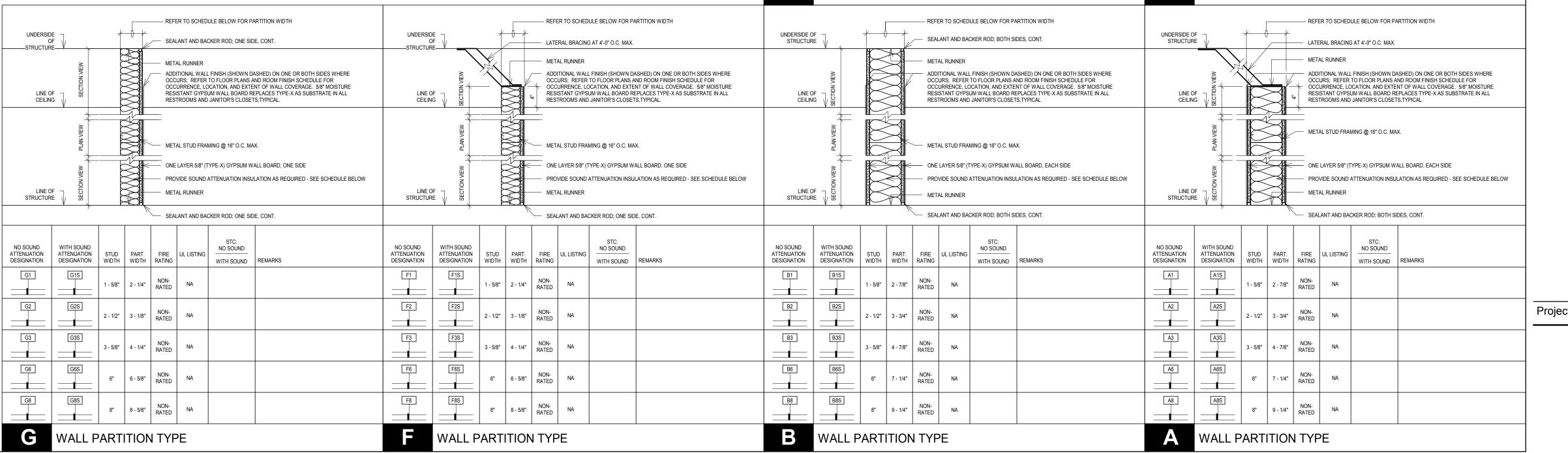
NO SOUND ATTENUATION DESIGNATION	WITH SOUND ATTENUATION DESIGNATION	CMU WIDTH	PART. WIDTH	FIRE RATING	UL LISTING	STC: NO SOUND WITH SOUND	REMARKS	NO SOUND ATTENUATION DESIGNATION	WITH SOUND ATTENUATION DESIGNATION	CMU WIDTH	PART. WIDTH	FIRE RATING	UL LISTING	STC: NO SOUND WITH SOUND	REMARKS	
N4	N4S	3 - 5/8"	3 - 5/8"	ONE HOUR	UL Des U905 or U906			M4	M4S	3 - 5/8"	3 - 5/8"	NON- RATED	NA			
N6	N6S	5 - 5/8"	5 - 5/8"	ONE HOUR	UL Des U905 or U906			M6	M6S	5 - 5/8"	5 - 5/8"	NON- RATED	NA			
N8	N8S	7 - 5/8"	7 - 5/8"	ONE HOUR	UL Des U905 or U906			M8	M8S	7 - 5/8"	7 - 5/8"	NON- RATED	NA			F
N12	N12S	11 - 5/8"	11 - 5/8"	ONE HOUR	UL Des U905 or U906			M12	M12S	11 - 5/8"	11 - 5/8"	NON- RATED	NA			<u>-'</u> '

ISSUANCE INDEX 08.20.18 PROJECT PHASE: 100% CONSTRUCTION DOCUMENTS - BP1

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NO.	DESCRIPTION	DATE					
·							

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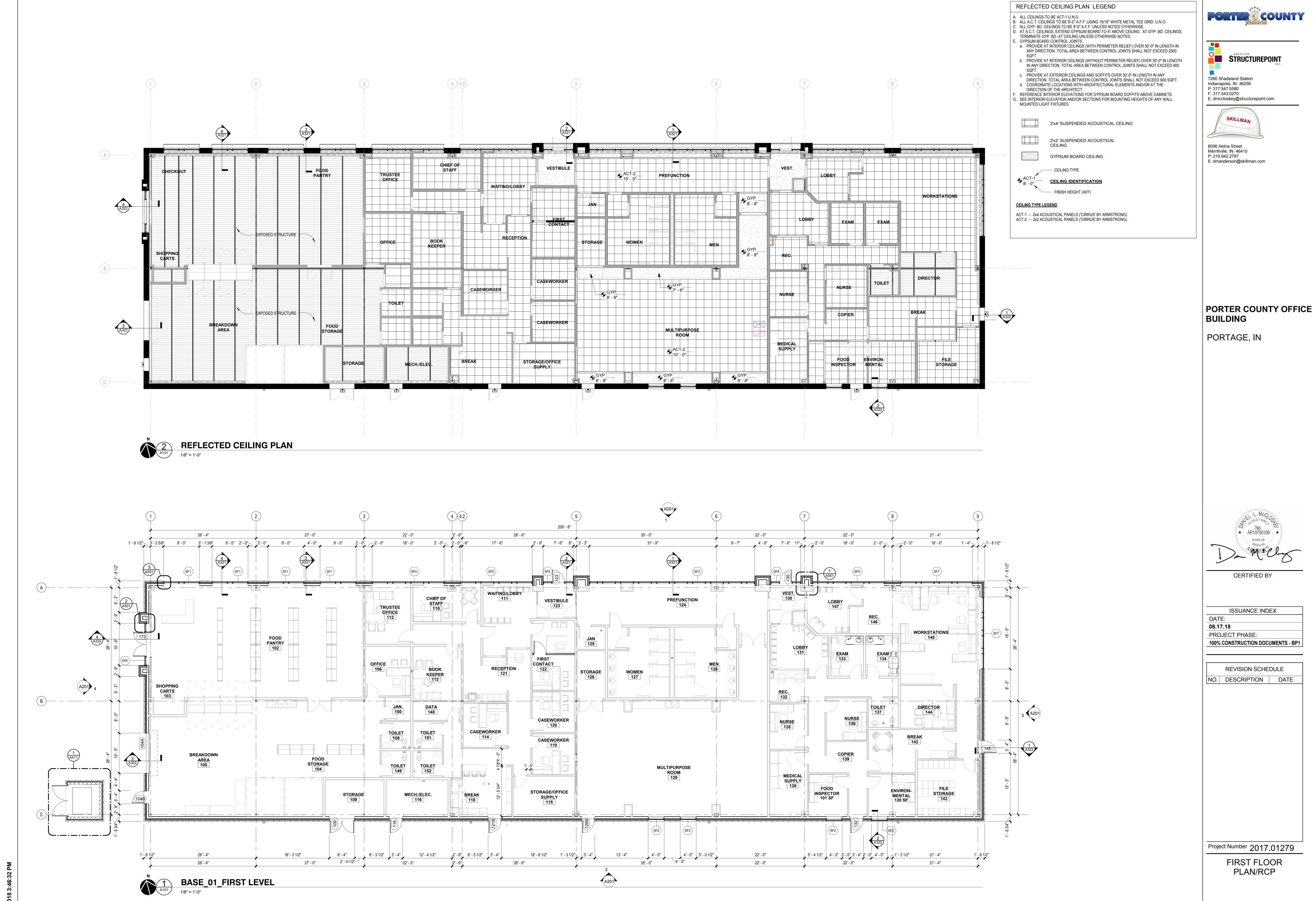
WALL TYPES



STRUCTURE

LINE OF CEILING

STRUCTURE



KEYED ROOF PLAN NOTES (NOTED WITH 1)

- 1 FULLY ADHERED MEMBRANE ROOFING SYSTEM (MEMBRANE OVER POLYISO ROOF INSULATION). INSTALL ROOF INSULATION IN MIN. 2 LAYERS WITH JOINTS OFFSET BY 12" MIN. PROVIDE TAPERED INSULATION TO ACHIEVE MIN. 1/4:12 SLOPE TO ENSURE POSITIVE DRAINAGE TO INTERNAL ROOF DRAIN LOCATIONS
- 2 PRE-FINISHED METAL COPING OVER PARAPET WALL
- 3 PRE-FINISHED METAL DOWNSPOUT
 4 LOCATION OF ROOFTOP HVAC UNITS SHOWN FOR REFERENCE ONLY.
 COORDINATE FINAL LOCATIONS WITH MECHANICAL & STRUCTURAL
 DRAWINGS. CREATE CRICKET AT UNIT HIGH SIDE TO ENSURE POSITIVE
 - DRAINAGE AROUND UNIT. PROVIDE ADDITIONAL CONCRETE WALKWAY
 PAD AT CONDENSATE DRAIN LINE.

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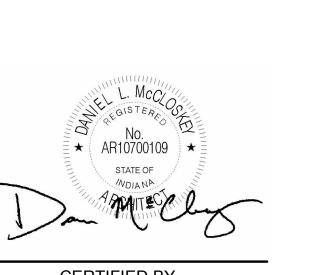
STRUCTUREPOINT

8006 Aetna Street Merrillville, IN 46410

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PORTER COUNTY OFFICE BUILDING

PORTAGE, IN



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DATE:

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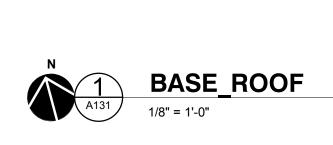
REVISION SCHEDULE

NO. DESCRIPTION DATE

NO. DESCRIPTION DATE

ROOF PLAN

Project Number 2017.01279



1/8" / 1'-0"

1/8" / 1'-0"

1/8" / 1'-0"

1/8" / 1'-0"







PORTER COUNTY

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SKILLMAN 8006 Aetna Street

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PORTER COUNTY OFFICE BUILDING

PORTAGE, IN

No.
AR10700109

STATE OF
WDIANA

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DATE:

08.17.18

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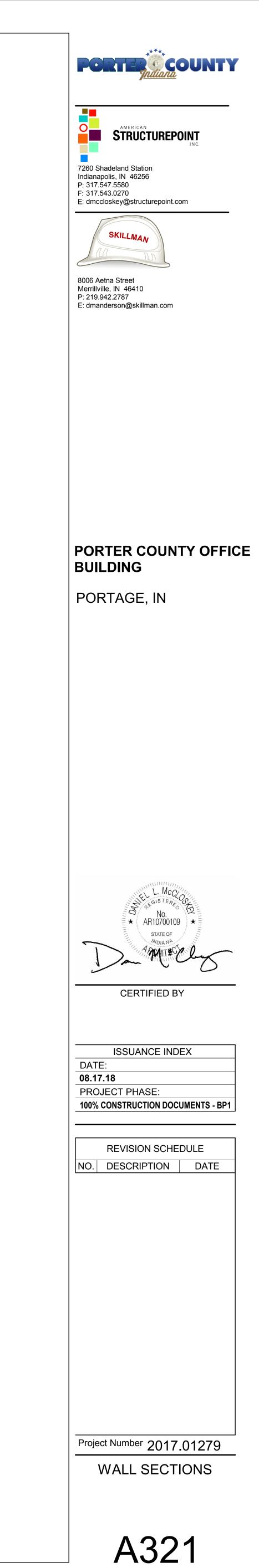
REVISION SCHEDULE

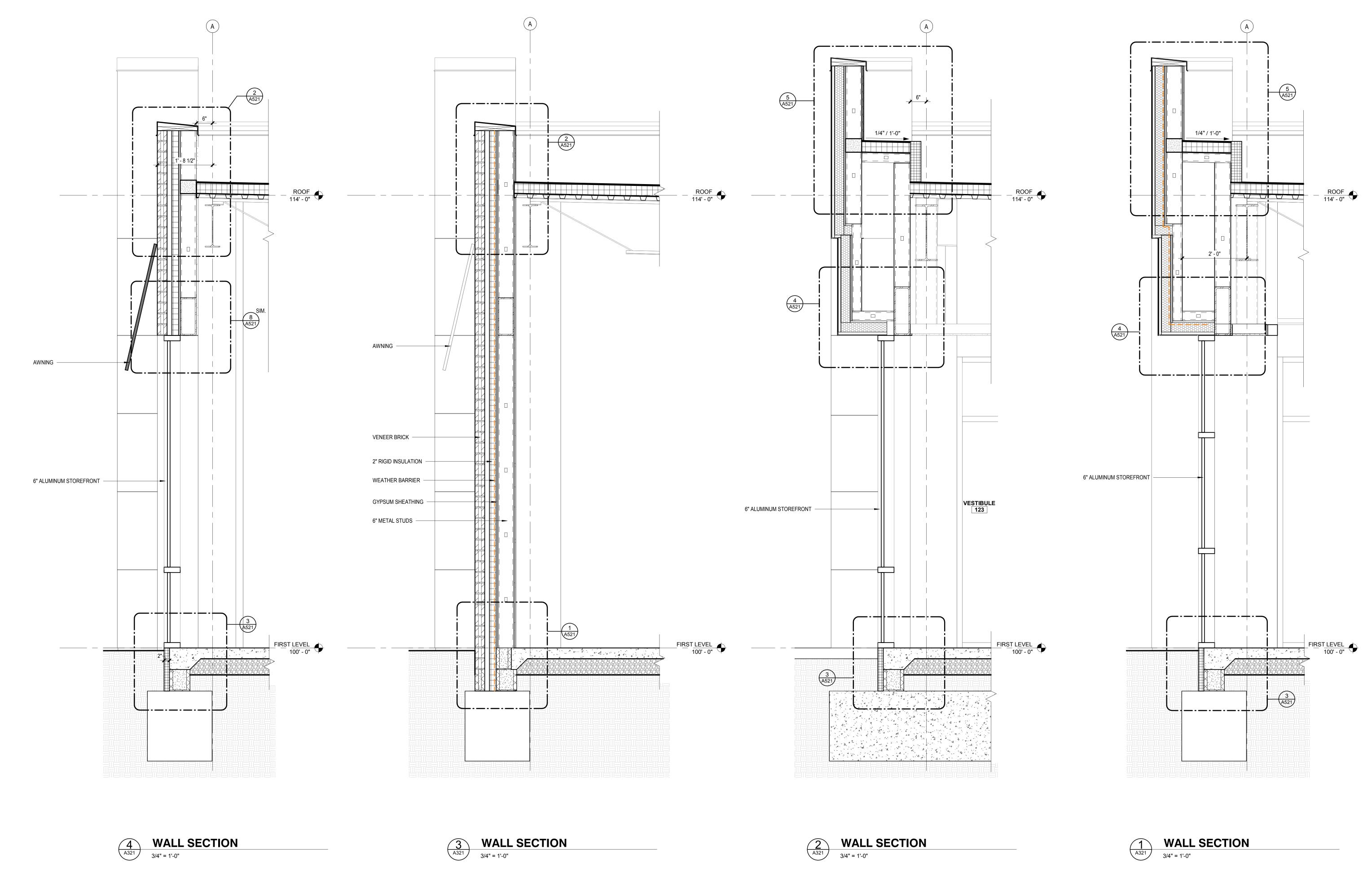
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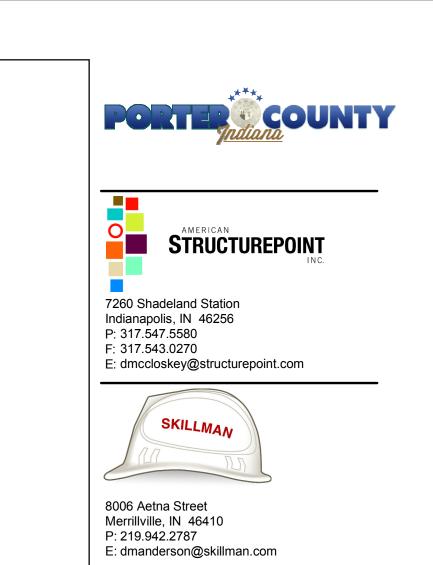
Project Number 2017.01279

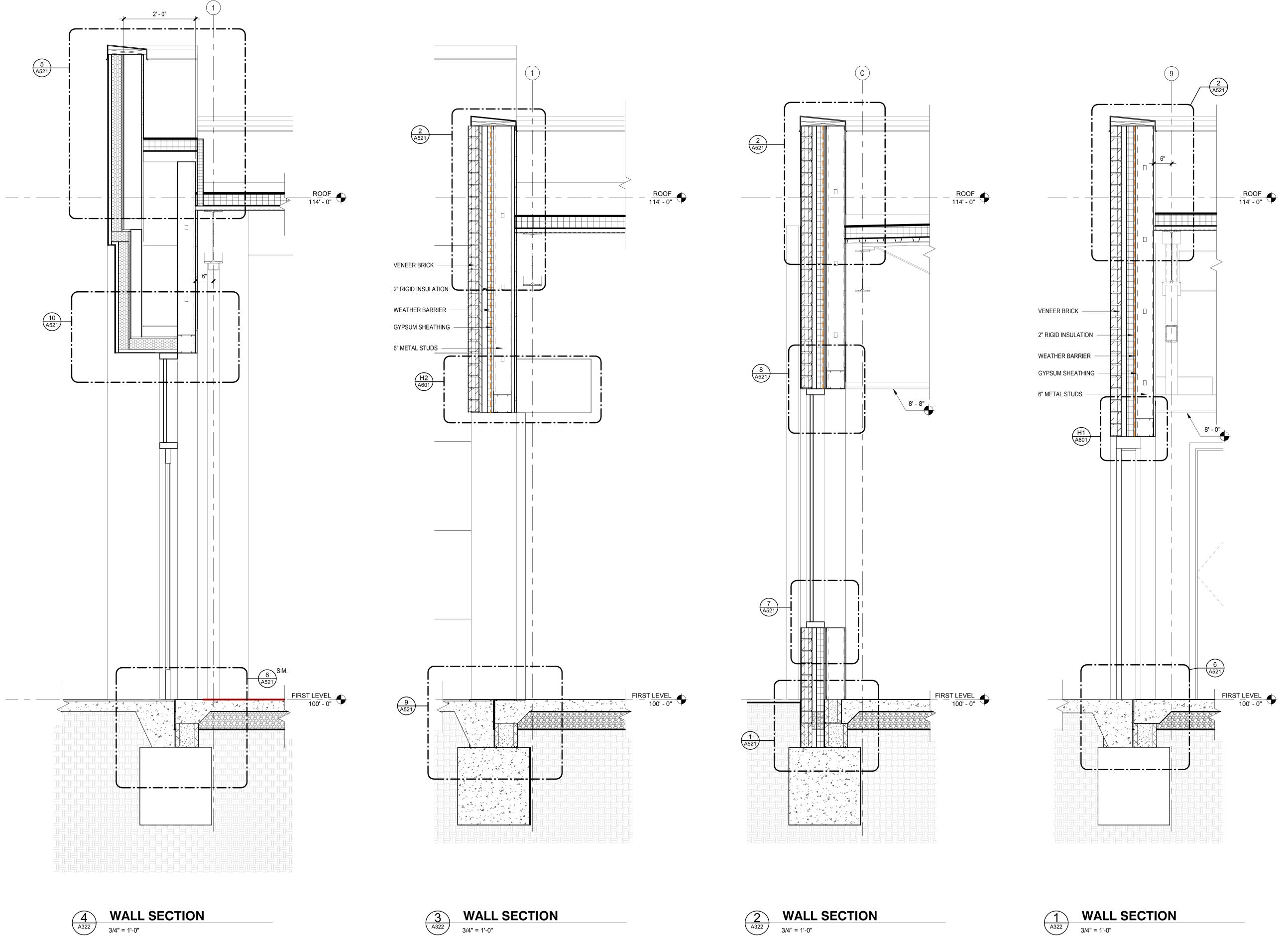
EXTERIOR ELEVATIONS

A201









PORTER COUNTY OFFICE BUILDING

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WALL SECTIONS





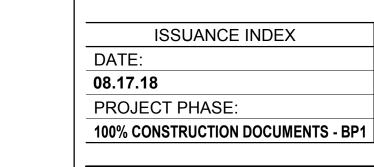


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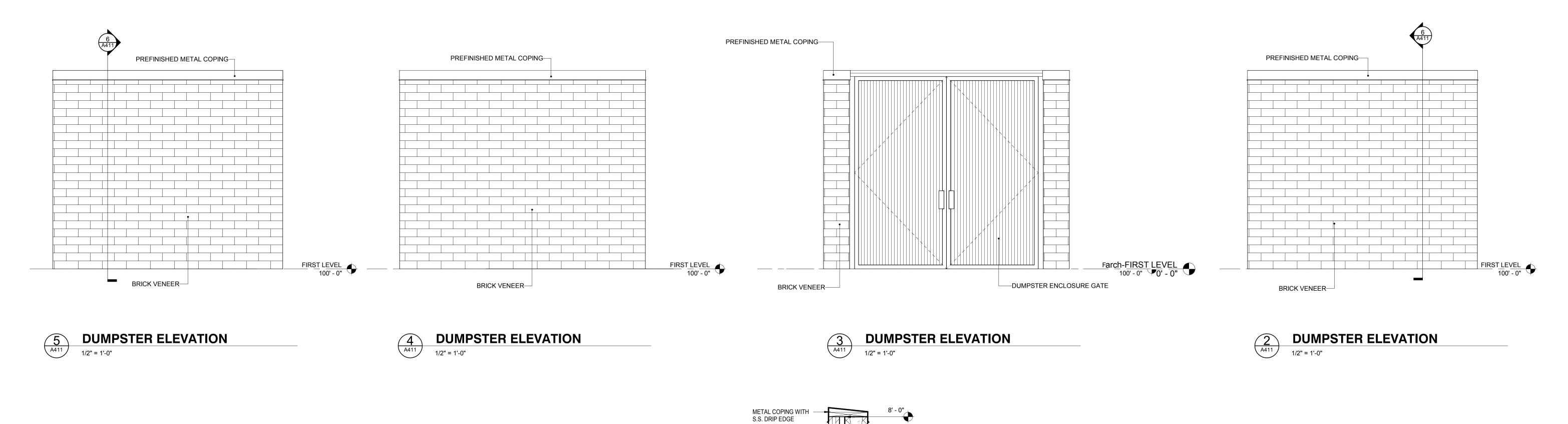


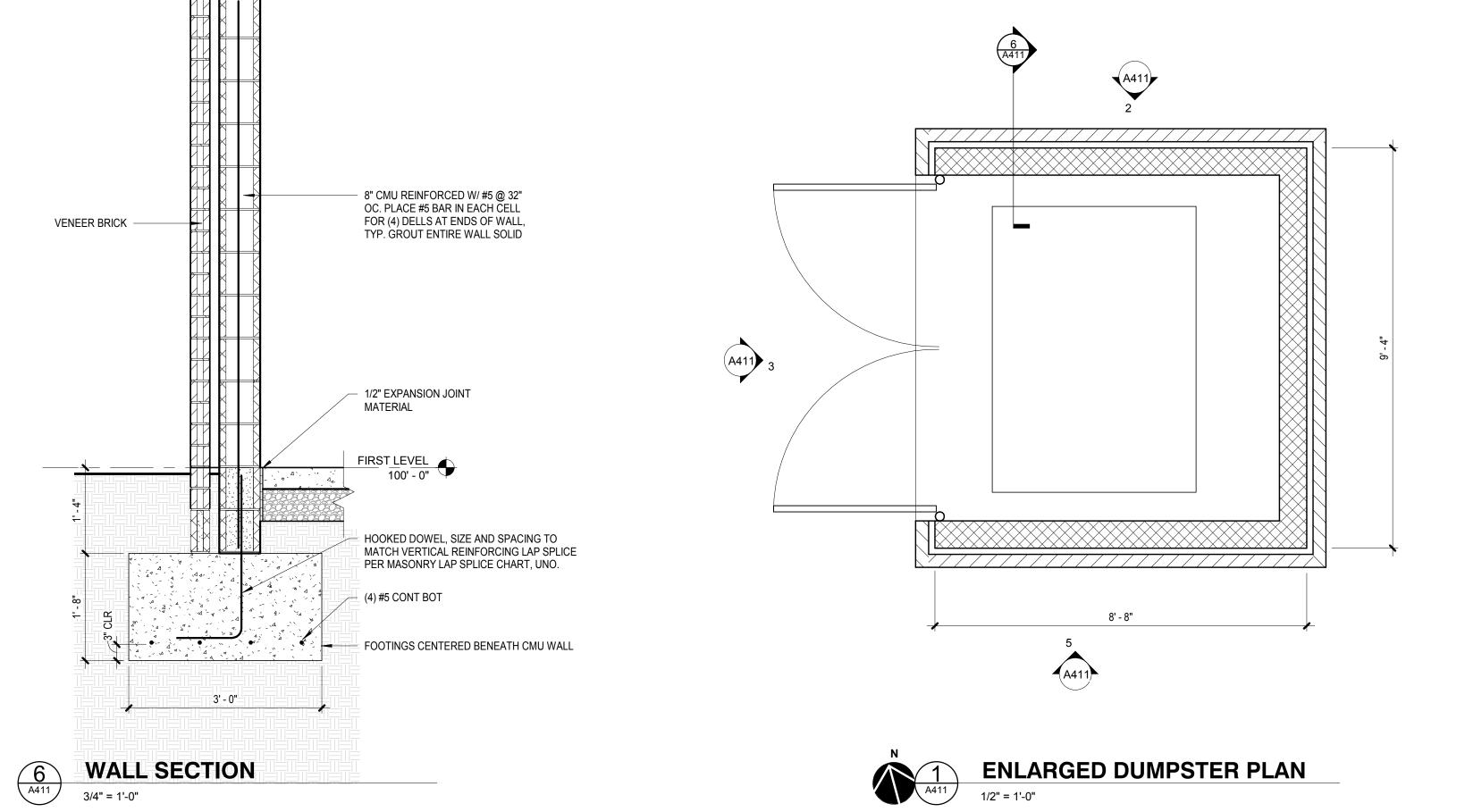
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NO.	DESCRIPTION	DATE

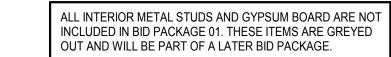
Project Number 2017.01279

DUMPSTER ENCLOSURE

A411

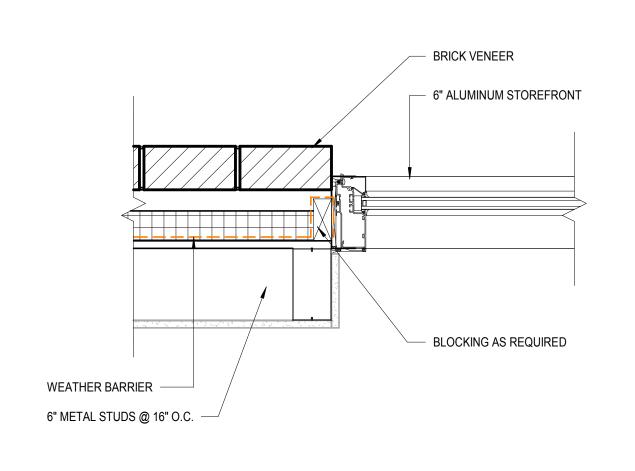




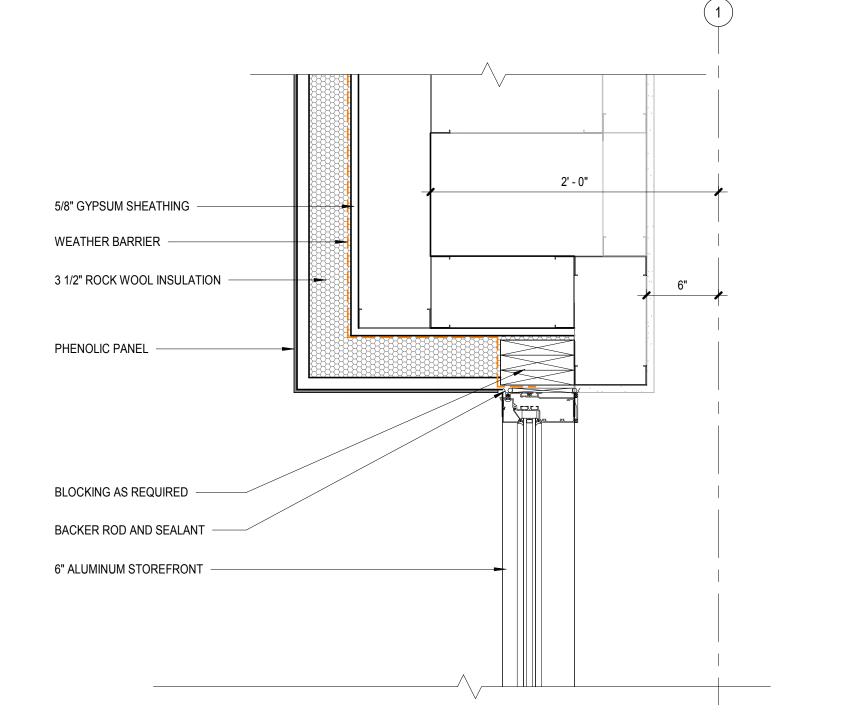




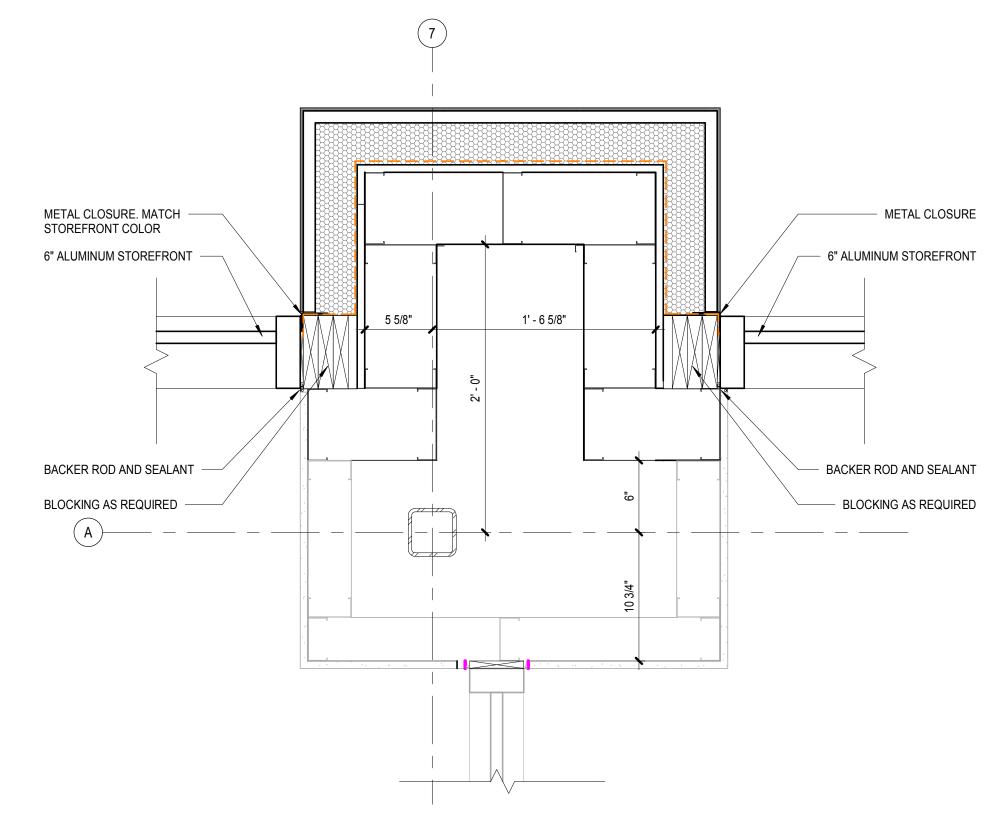




PLAN DETAIL







1 PLAN DETAIL
1 1/2" = 1'-0"



PORTAGE, IN



	ISSUANCE IND	EX
DAT	E:	
08.1	7.18	
PRC	JECT PHASE:	
100%	CONSTRUCTION DOC	UMENTS - BP'
	REVISION SCHE	DULE
NO	DESCRIPTION	DATE

	REVISION SCHE	DULE
10.	DESCRIPTION	DATE

PLAN DETAILS

Project Number 2017.01279

A501

